APPENDIX B: LIQUEFACTION ASSESSMENT RESULTS

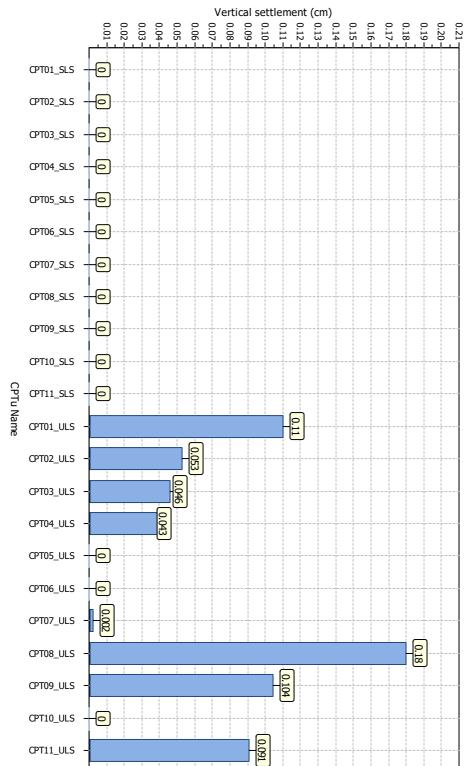




Project title : 195340402

Location: 131 Otaihanga Rd, Paraparaumu

Overall vertical settlements report

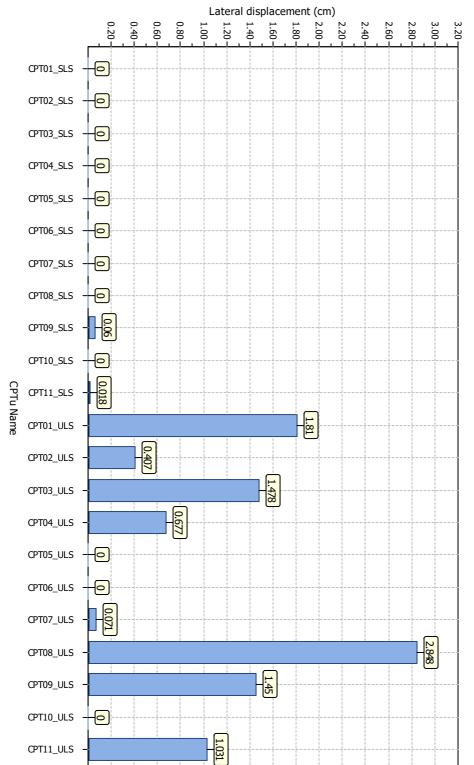




Project title: 195340402

Location: 131 Otaihanga Rd, Paraparaumu

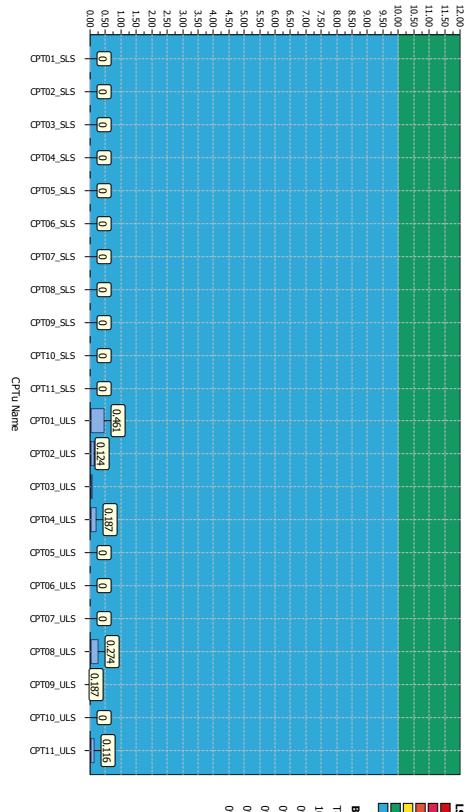
Overall lateral displacements report



Project title: 195340402 Location: 131 Otaihanga

Location: 131 Otaihanga Rd, Paraparaumu

Overall Liquefaction Severity Number report



LSN value

■ Severe damage

Major expression of liquefaction Moderate to severe exp. of liquefaction Moderate expression of liquefaction Minor expression of liquefaction

Basic statistics

Little to no expression of liquefaction

Total CPT number: 22
100% little liquefaction
0% minor liquefaction
0% moderate liquefaction
0% moderate to major liquefaction
0% major liquefaction
0% severe liquefaction

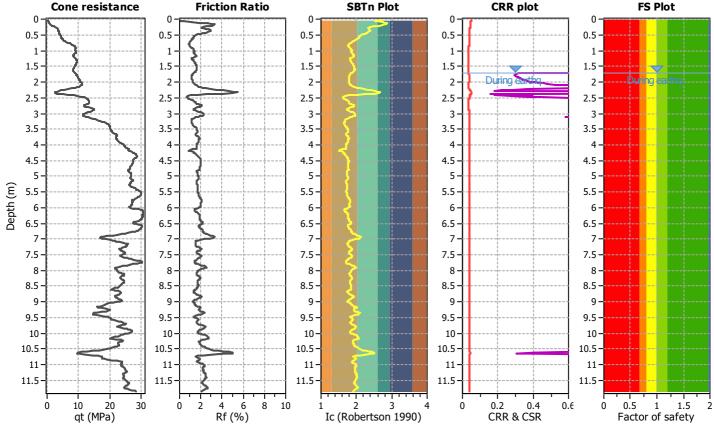


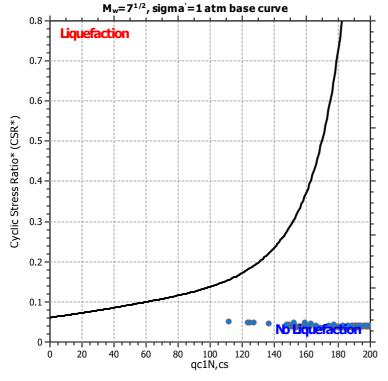
Project title: 195340402 Location: 131 Otaihanga Rd, Paraparaumu

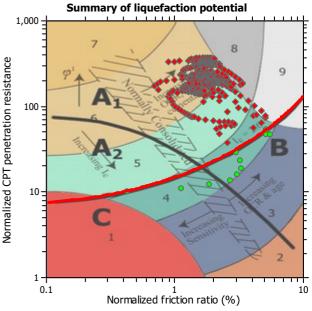
CPT file: CPT01_SLS

Input parameters and analysis data

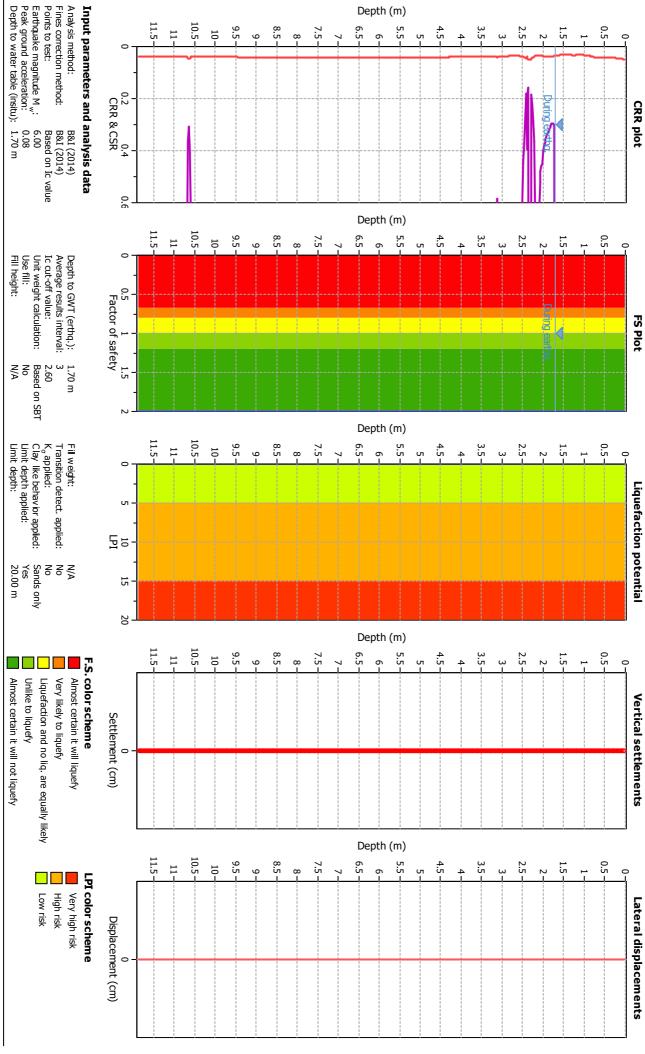
A naly sis method: B&I (2014) G.W.T. (in-situ): 1.70 m Use fill: No Clay like behavior Fines correction method: B&I (2014) G.W.T. (earthq.): 1.70 m Fill height: N/A applied: Sands only Points to test: Based on Ic value Average results interval: 3 Fill weight: N/A Limit depth applied: Yes Earthquake magnitude M w: 6.00 Ic cut-off value: 2.60 Trans. detect. applied: No Limit depth: 20.00 m Peak ground acceleration: Unit weight calculation: Based on SBT K_{σ} applied: No MSF method: Method based







Zone A_1 : Cyclic liquefaction likely depending on size and duration of cyclic loading Zone A_2 : Cyclic liquefaction and strength loss likely depending on loading and ground geometry



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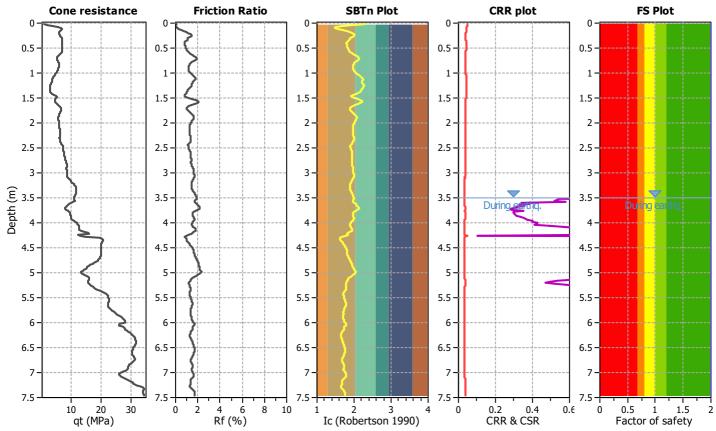


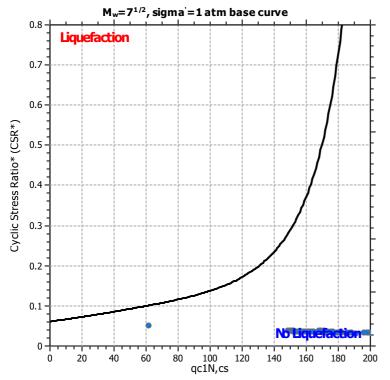
Project title: 195340402 Location: 131 Otaihanga Rd, Paraparaumu

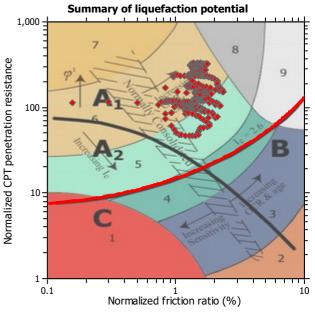
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Input parameters and analysis data

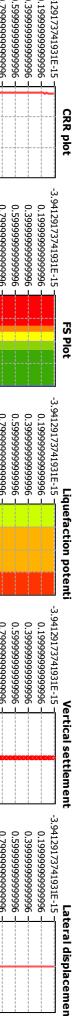
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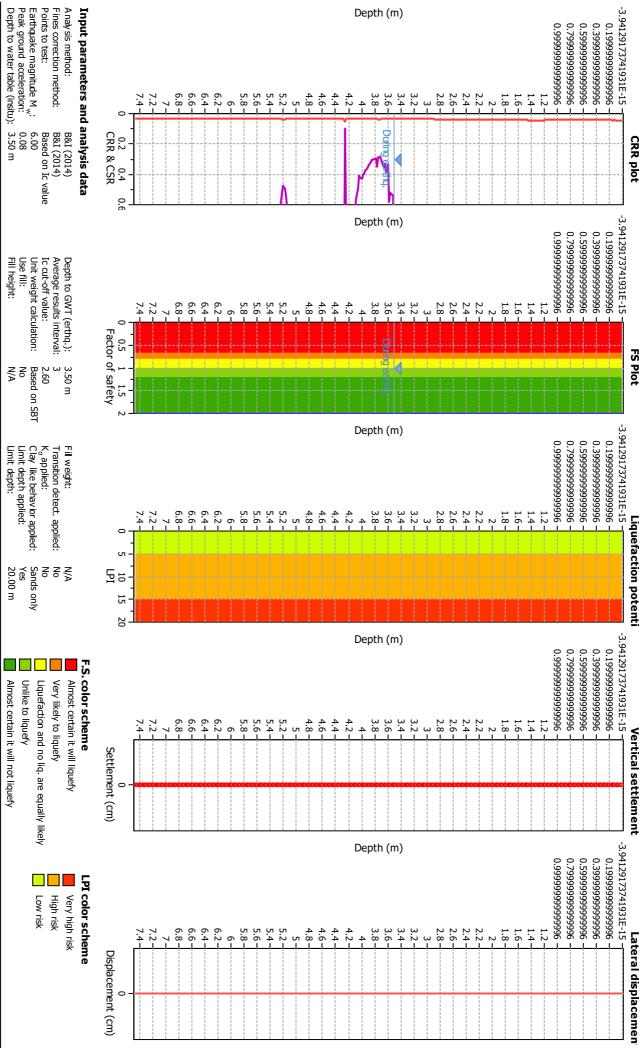






Zone A_1 : Cyclic liquefaction likely depending on size and duration of cyclic loading Zone A_2 : Cyclic liquefaction and strength loss likely depending on loading and ground geometry





Project file: S:\Projects\195340402_131 Otaihanga Rd_Mr Mansel\\04 Assessment\131 Otaihanga Rd\CLiq\195340402_CLiq.clq CLiq v.3.0.2.4 - CPT Liquefaction Assessment Software - Report created on: 15/04/2020, 5:44:31 PM

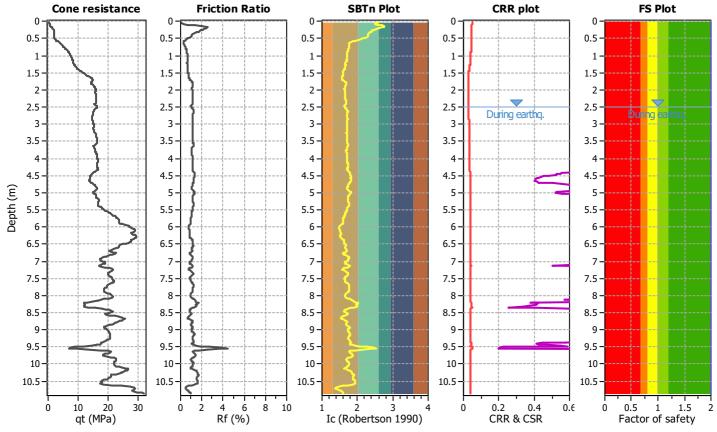


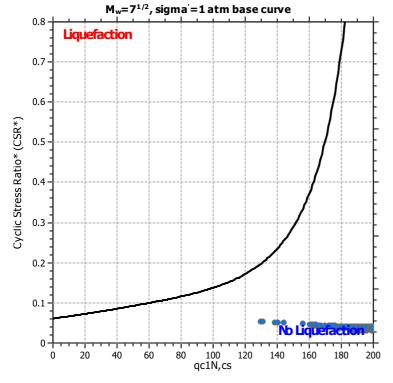
Project title: 195340402 Location: 131 Otaihanga Rd, Paraparaumu

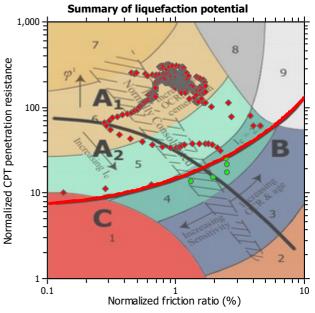
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Input parameters and analysis data

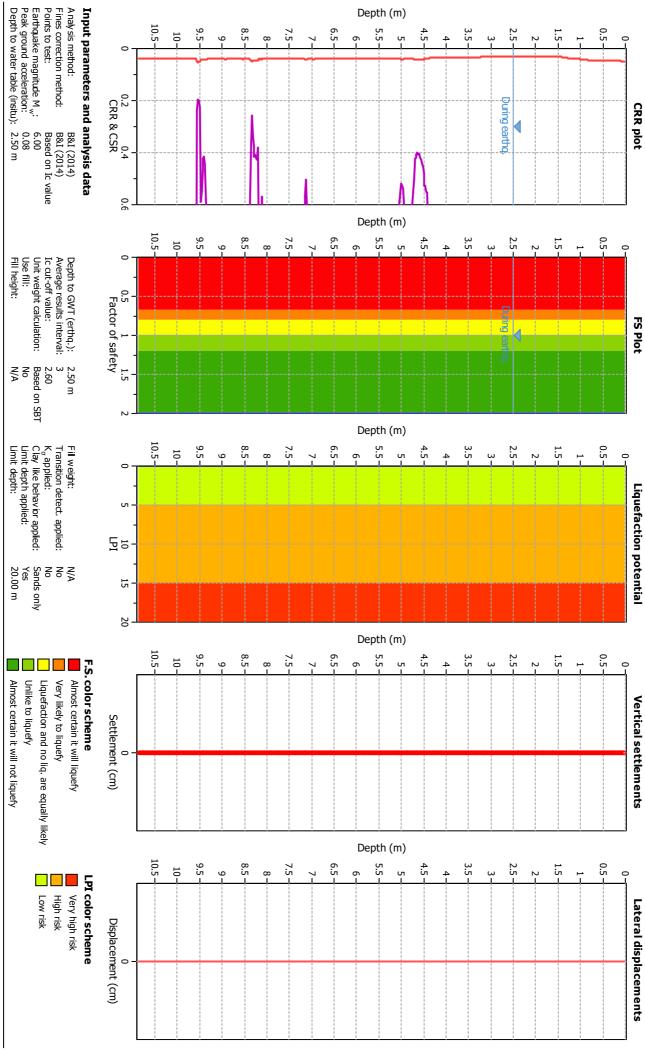
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Zone A_1 : Cyclic liquefaction likely depending on size and duration of cyclic loading Zone A_2 : Cyclic liquefaction and strength loss likely depending on loading and ground geometry



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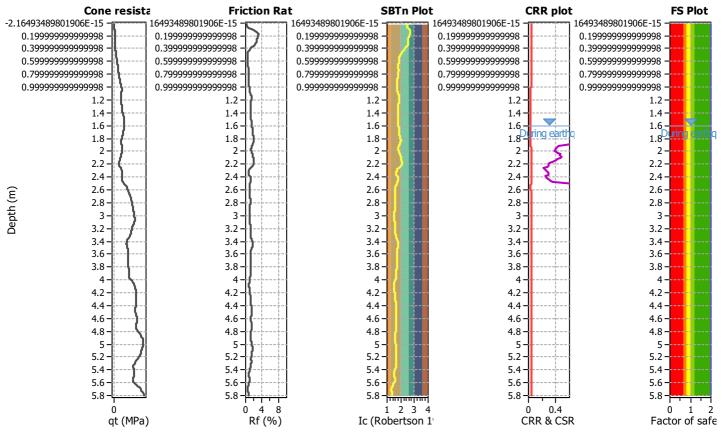


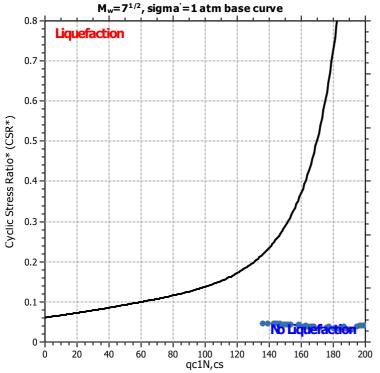
Project title: 195340402 Location: 131 Otaihanga Rd, Paraparaumu

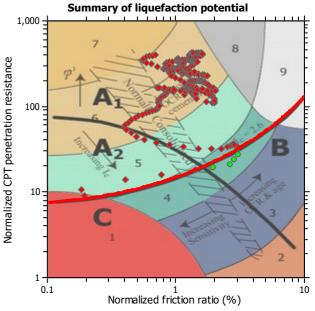
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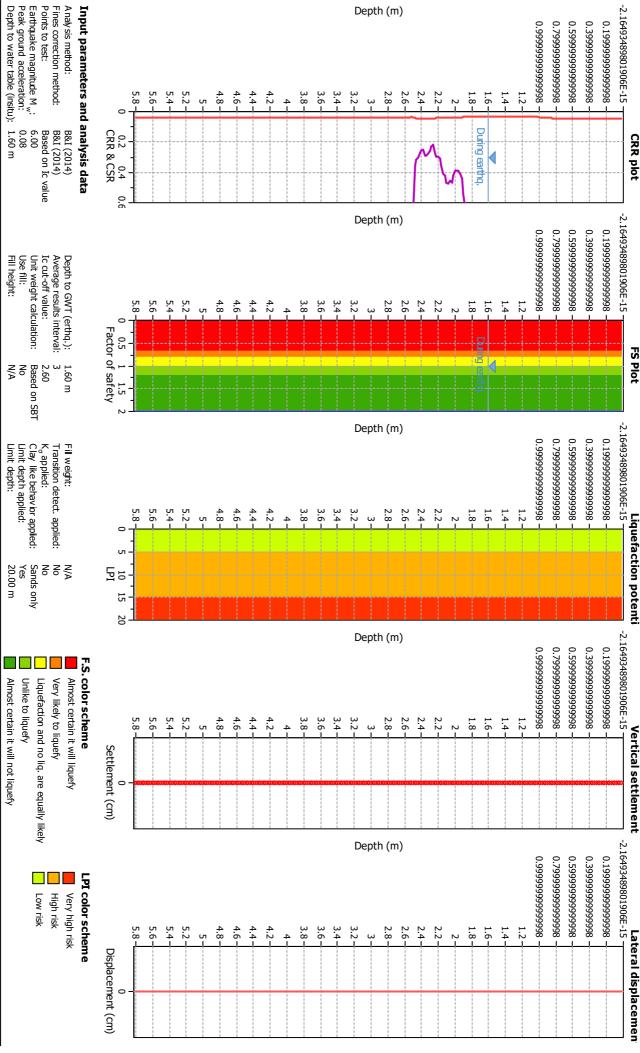




Zone A₁: Cyclic liquefaction likely depending on size and duration of cyclic loading Zone A₂: Cyclic liquefaction and strength loss likely depending on loading and ground

CRR plot **FS Plot** Liquefaction potenti Vertical settlement

Liquefaction analysis overall plots



Project file: S:\Projects\195340402_131 Otaihanga Rd_Mr Mansel\\04 Assessment\131 Otaihanga Rd\CLiq\195340402_CLiq.clq CLiq v.3.0.2.4 - CPT Liquefaction Assessment Software - Report created on: 15/04/2020, 5:44:32 PM



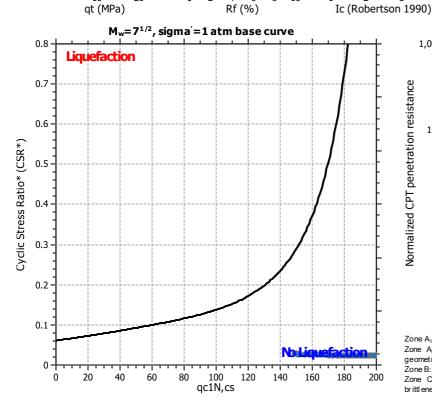
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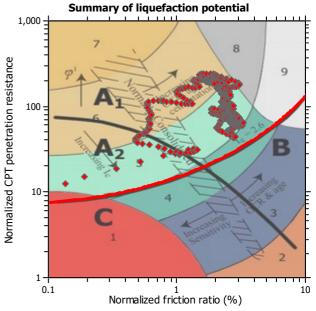
CPT file: CPT05_SLS

Input parameters and analysis data

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Cone resistance Friction Ratio SBTn Plot CRR plot **FS Plot** 0 0 0 1 2 2 3 3 3 3 3 5 5 6 6 7 7 8 9 9 10 10 10 10 11 12 12 12 12 13 During earthq. During. 14 14 15 15 15 16 16 16 16 10 0.5 1.5 0.2 0.4 0.6





CRR & CSR

Zone A₁: Cyclic liquefaction likely depending on size and duration of cyclic loading Zone A₂: Cyclic liquefaction and strength loss likely depending on loading and ground

Zone B: Liquefaction and post-earthquake strength loss unlikely, check cyclic softening Zone C: Cyclic liquefaction and strength loss possible depending on soil plasticity, brittleness/sensitivity, strain to peak undrained strength and ground geometry

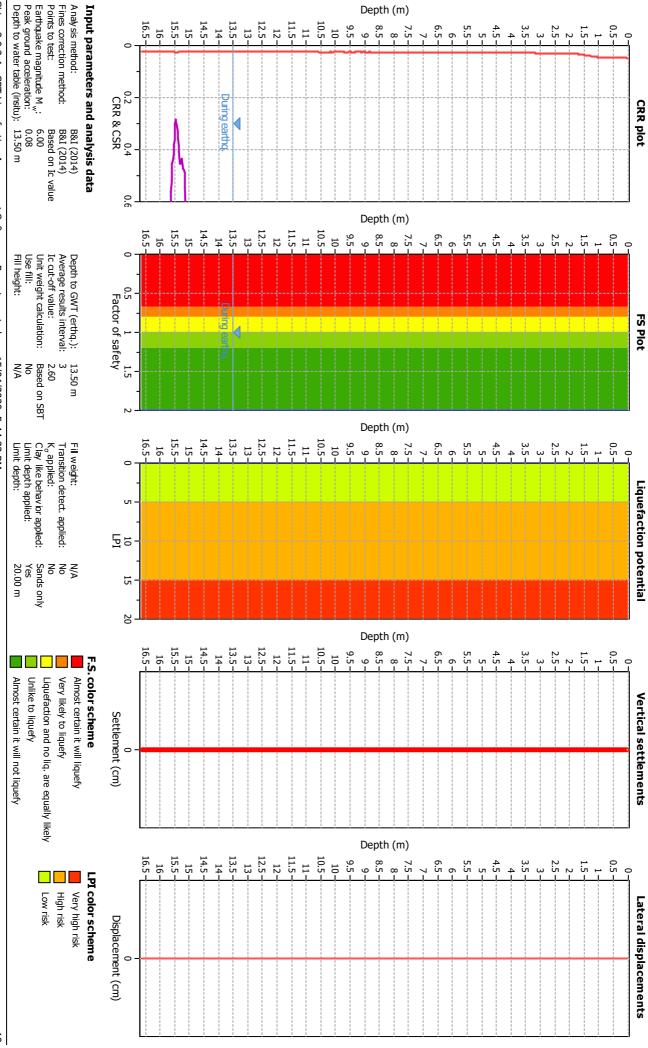
Sands only

Method based

20.00 m

Yes

Factor of safety



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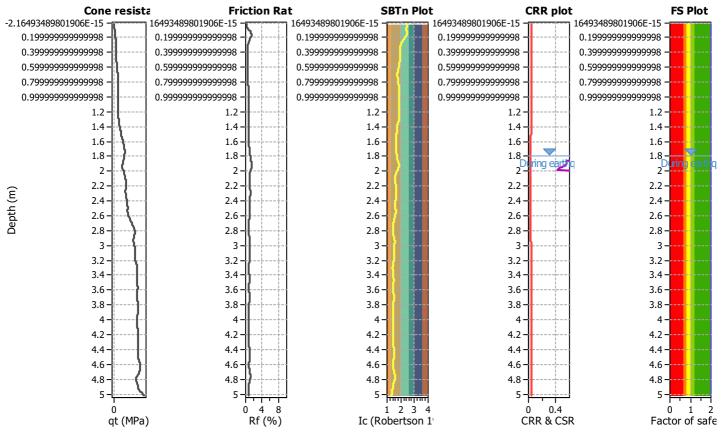


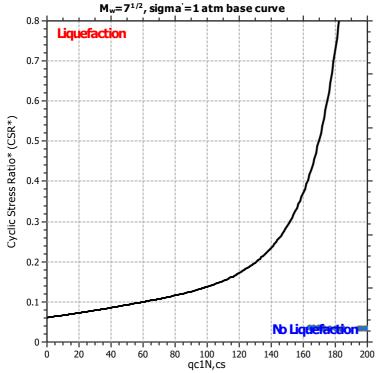
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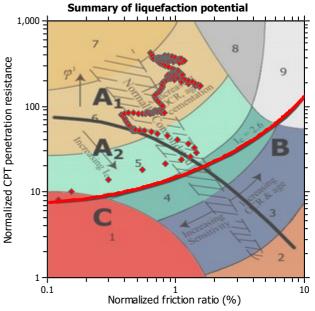
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Input parameters and analysis data

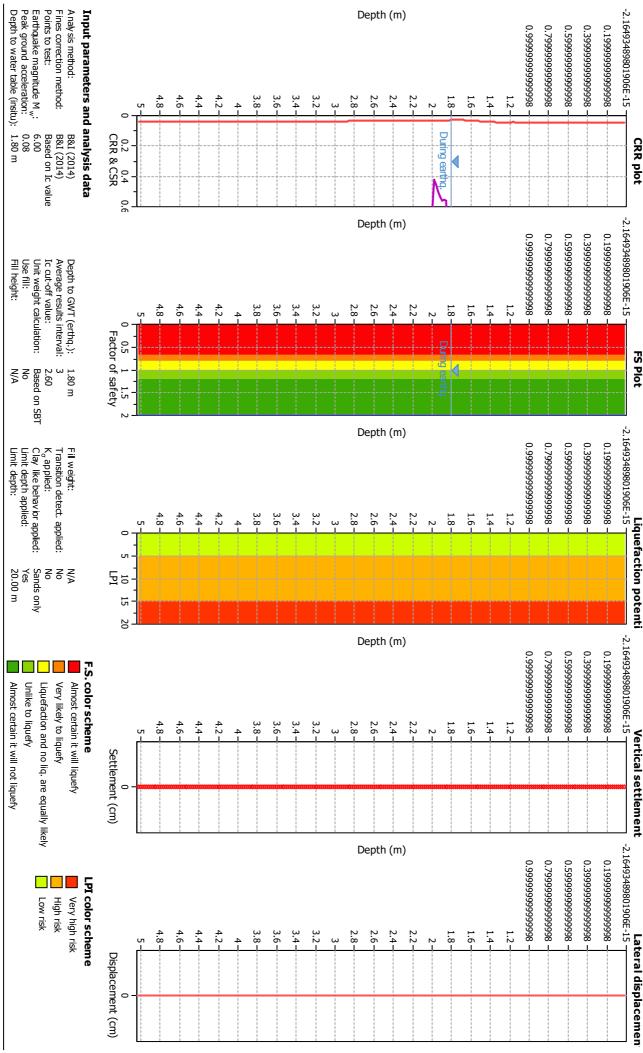
A naly sis method: B&I (2014) G.W.T. (in-situ): 1.80 m Use fill: No Clay like behavior Fines correction method: B&I (2014) G.W.T. (earthq.): 1.80 m Fill height: N/A applied: Sands only Points to test: Based on Ic value Average results interval: 3 Fill weight: N/A Limit depth applied: Yes Earthquake magnitude M w: 6.00 Ic cut-off value: 2.60 Trans. detect. applied: No Limit depth: 20.00 m Peak ground acceleration: Unit weight calculation: Based on SBT K_{σ} applied: MSF method: Method based No







Zone A_1 : Cyclic liquefaction likely depending on size and duration of cyclic loading Zone A_2 : Cyclic liquefaction and strength loss likely depending on loading and ground geometry



CLiq v.3.0.2.4 - CPT Liquefaction Assessment Software - Report created on: 15/04/2020, 5:44:33 PM Project file: S:\Projects\195340402_131 Otaihanga Rd\CLiq\195340402_CLiq.clq

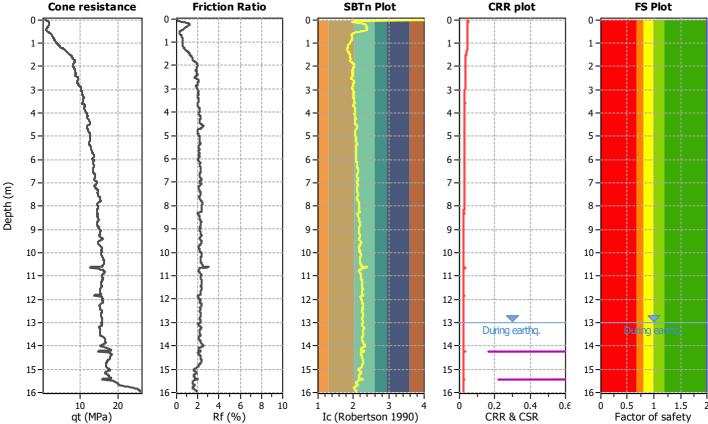


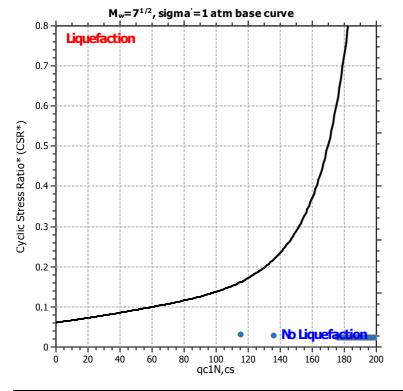
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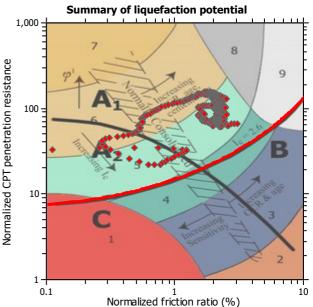
CPT file: CPT07_SLS

Input parameters and analysis data

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Zone A_1 : Cyclic liquefaction likely depending on size and duration of cyclic loading Zone A_2 : Cyclic liquefaction and strength loss likely depending on loading and ground geometry

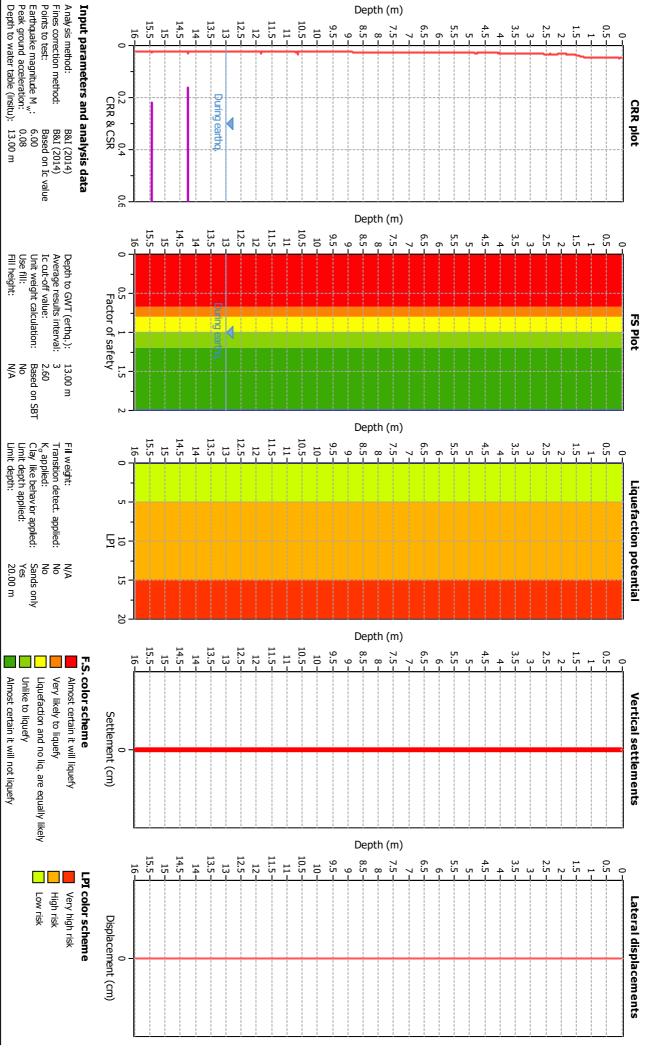
Zone B: Liquefaction and post-earthquake strength loss unlikely, check cyclic softening Zone C: Cyclic liquefaction and strength loss possible depending on soil plasticity, brittleness/sensitivity, strain to peak undrained strength and ground geometry

Sands only

Method based

20.00 m

Yes



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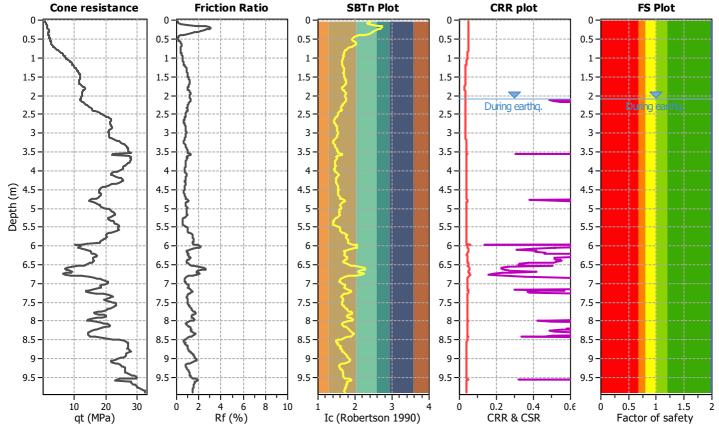


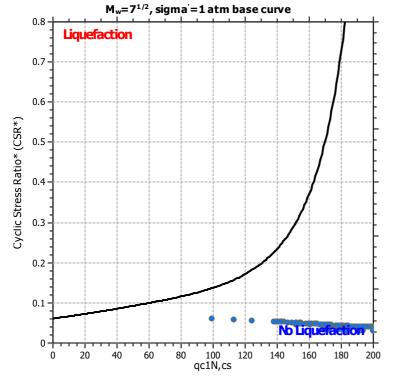
Project title: 195340402 Location: 131 Otaihanga Rd, Paraparaumu

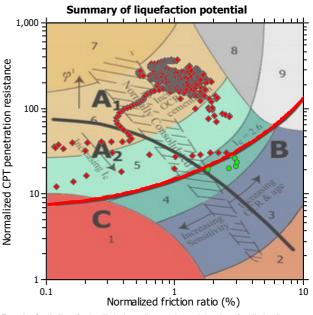
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Input parameters and analysis data

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Zone A_1 : Cyclic liquefaction likely depending on size and duration of cyclic loading Zone A_2 : Cyclic liquefaction and strength loss likely depending on loading and ground geometry

Depth (m) Depth to water table (insitu): Earthquake magnitude M_w: Fines correction method: A naly sis method: Input parameters and analysis data Peak ground acceleration: Points to test: 1.5-8.5 6.5 5.5 4. 5. 3.5-2.5-0.5 7.5-0.2 0.4 CRR & CSR puring earthq **CRR plot** 6.00 0.08 2.10 m B&I (2014) B&I (2014) Based on Ic value 0.6 Depth (m) 6.5 5.5 4.5 0.5 9.5 7.5-3.5 2.5 1.5 6 9œ 2-Unit weight calculation: Use fill: Fill height: Average results interval: Depth to GWT (erthq.): Ic cut-off value: 0.5 Factor of safety **FS Plot** 2.10 m 3 2.60 1.5 Νď Based on SBT Depth (m) 4.5 5.5 6.5 3.5 2.5 7.5 1.5 K_{σ} applied: Clay like behavior applied: 6 Limit depth applied: Fill weight: œ ω 2-Transition detect. applied: **Liquefaction potential** G N/A No No Sands only Yes 20.00 m 15 20 Depth (m) 8.5 6.5 4. 5. 7.5 3.5 2.5 1.5 0 F.S. color scheme Almost certain it will liquefy Very likely to liquefy Liquefaction and no liq. are equally likely Almost certain it will not liquefy Unlike to liquery Vertical settlements Settlement (cm) Depth (m) 9.5 6.5 . 4. 5 0.5 3.5-2.5 8.5 7.5 5.5 1.5-LPI color scheme High risk Low risk Very high risk Lateral displacements Displacement (cm)

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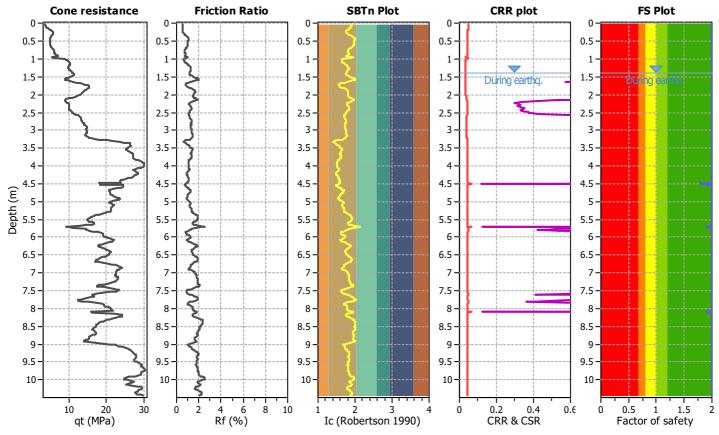


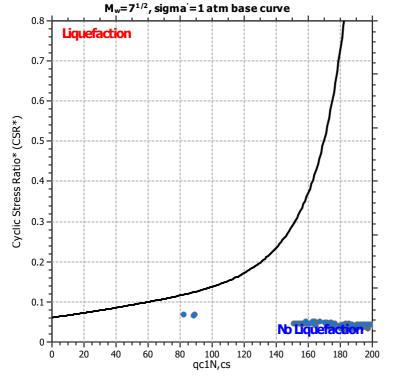
Project title: 195340402 Location: 131 Otaihanga Rd, Paraparaumu

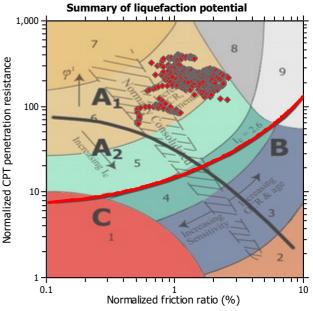
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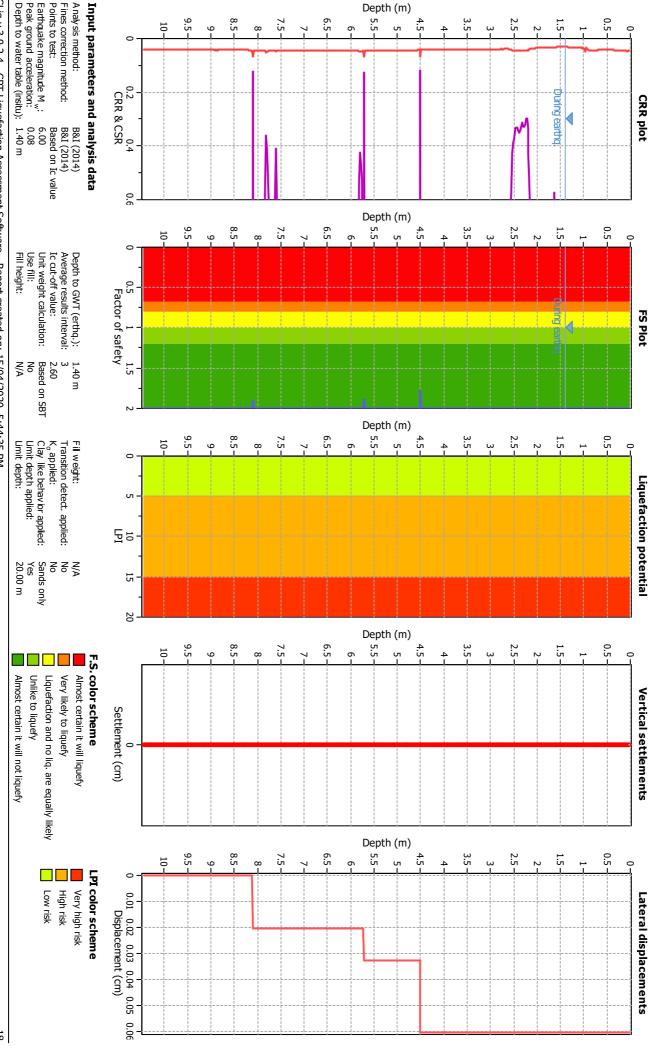
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Zone A_1 : Cyclic liquefaction likely depending on size and duration of cyclic loading Zone A_2 : Cyclic liquefaction and strength loss likely depending on loading and ground geometry



CLiq v.3.0.2.4 - CPT Liquefaction Assessment Software - Report created on: 15/04/2020, 5:44:35 PM Project file: S:\Projects\195340402_131 Otaihanga Rd\CLiq\195340402_CLiq.clq

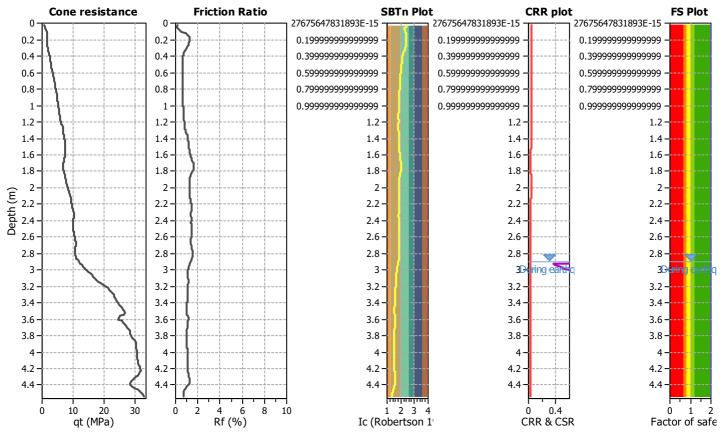


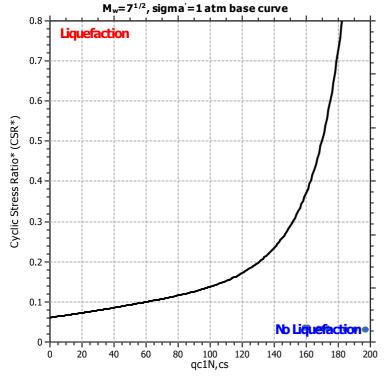
Project title: 195340402 Location: 131 Otaihanga Rd, Paraparaumu

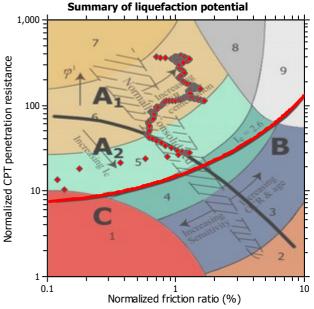
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Input parameters and analysis data

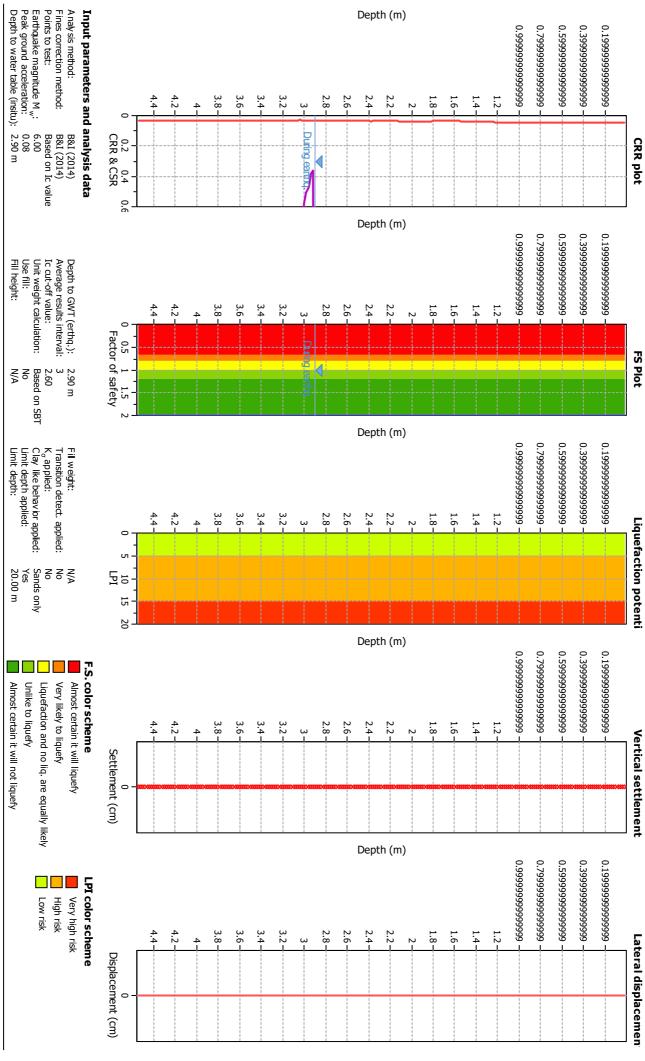
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Zone A_1 : Cyclic liquefaction likely depending on size and duration of cyclic loading Zone A_2 : Cyclic liquefaction and strength loss likely depending on loading and ground geometry



CLiq v.3.0.2.4 - CPT Liquefaction Assessment Software - Report created on: 15/04/2020, 5:44:35 PM Project file: S:\Projects\195340402_131 Otaihanga Rd_Mr Mansel\04 Assessment\131 Otaihanga Rd\CLiq\195340402_CLiq.dq

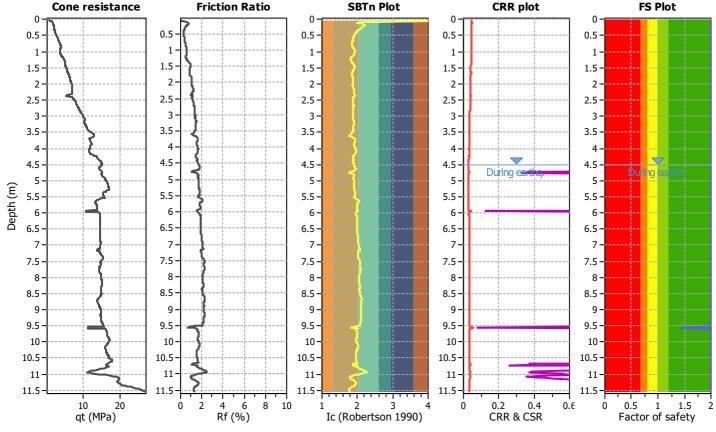


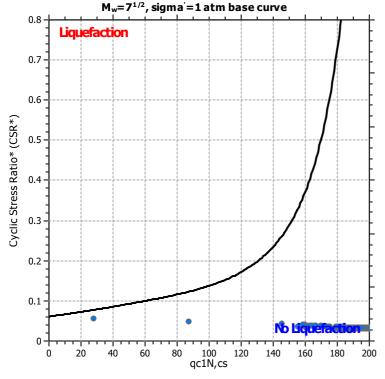
Project title: 195340402 Location: 131 Otaihanga Rd, Paraparaumu

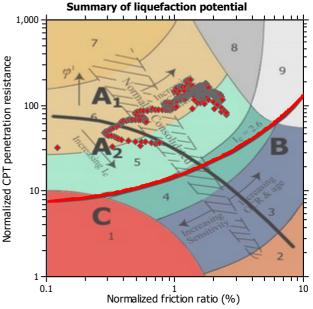
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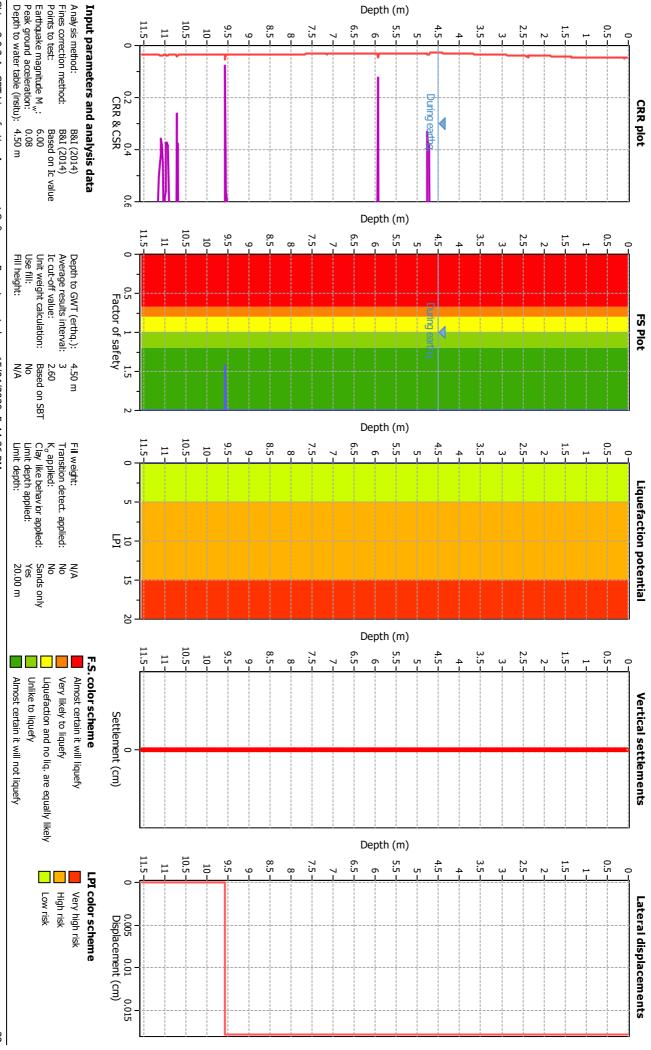
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Zone A_1 : Cyclic liquefaction likely depending on size and duration of cyclic loading Zone A_2 : Cyclic liquefaction and strength loss likely depending on loading and ground geometry



CLiq v.3.0.2.4 - CPT Liquefaction Assessment Software - Report created on: 15/04/2020, 5:44:36 PM Project file: S:\Projects\195340402_131 Otaihanga Rd\CLiq\195340402_CLiq.clq

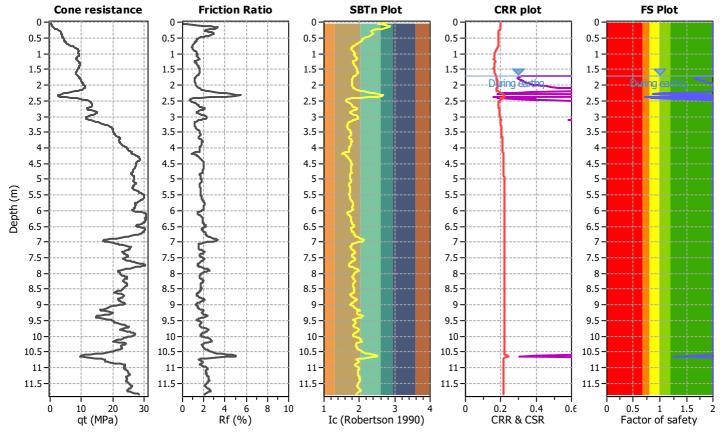


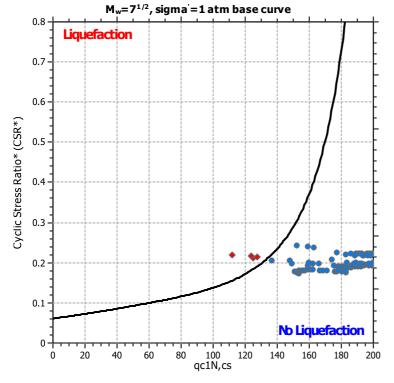
Project title: 195340402 Location: 131 Otaihanga Rd, Paraparaumu

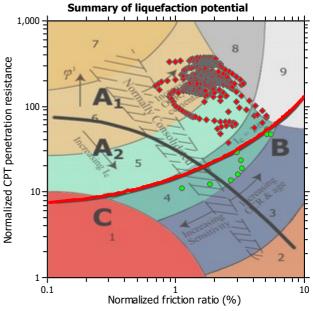
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Input parameters and analysis data

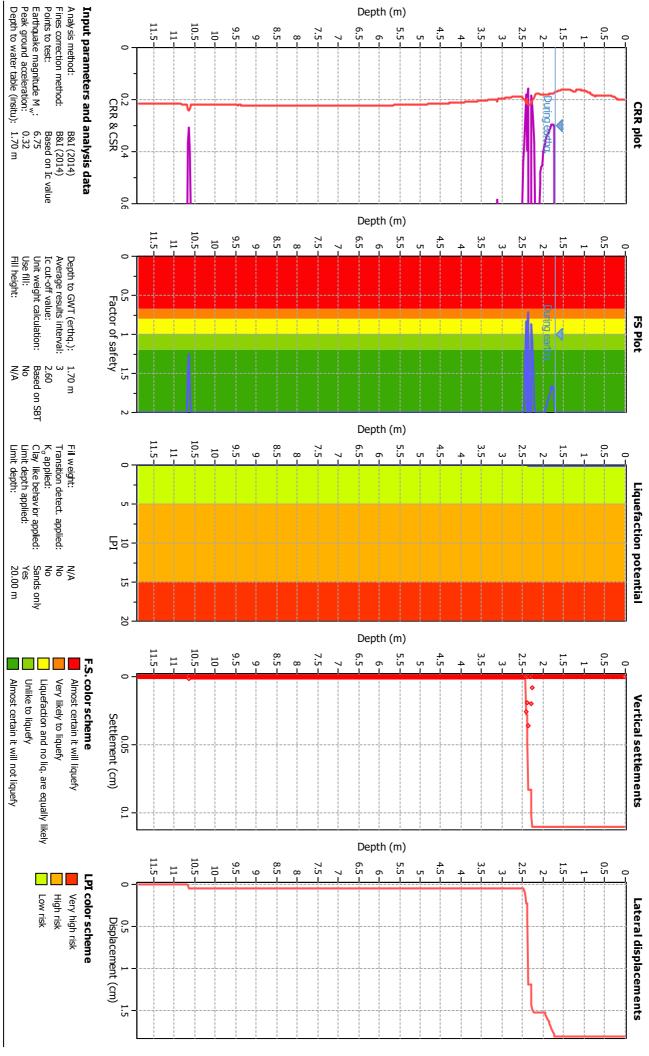
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Zone A₁: Cyclic liquefaction likely depending on size and duration of cyclic loading Zone A₂: Cyclic liquefaction and strength loss likely depending on loading and ground



CLiq v.3.0.2.4 - CPT Liquefaction Assessment Software - Report created on: 15/04/2020, 5:44:37 PM Project file: S:\Projects\195340402_131 Otaihanga Rd\CLiq\195340402_CLiq.clq

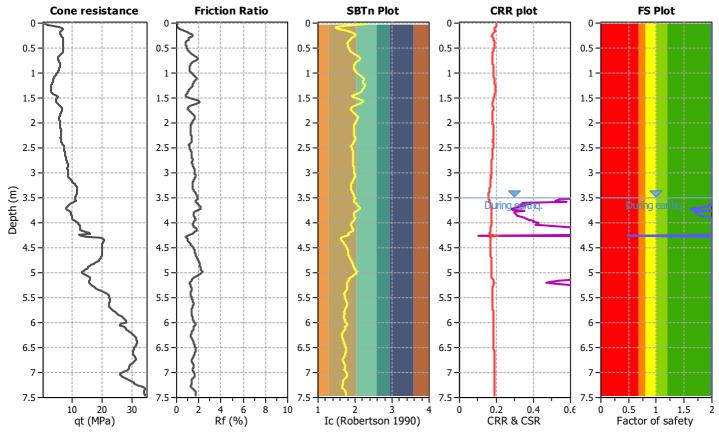


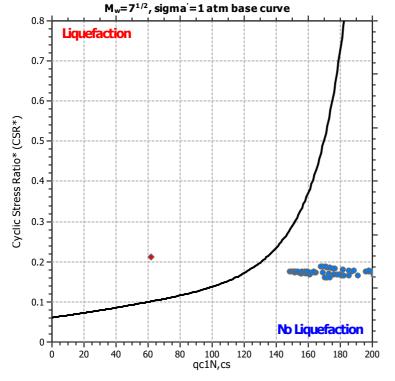
Project title: 195340402 Location: 131 Otaihanga Rd, Paraparaumu

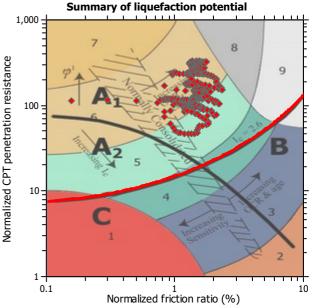
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Input parameters and analysis data

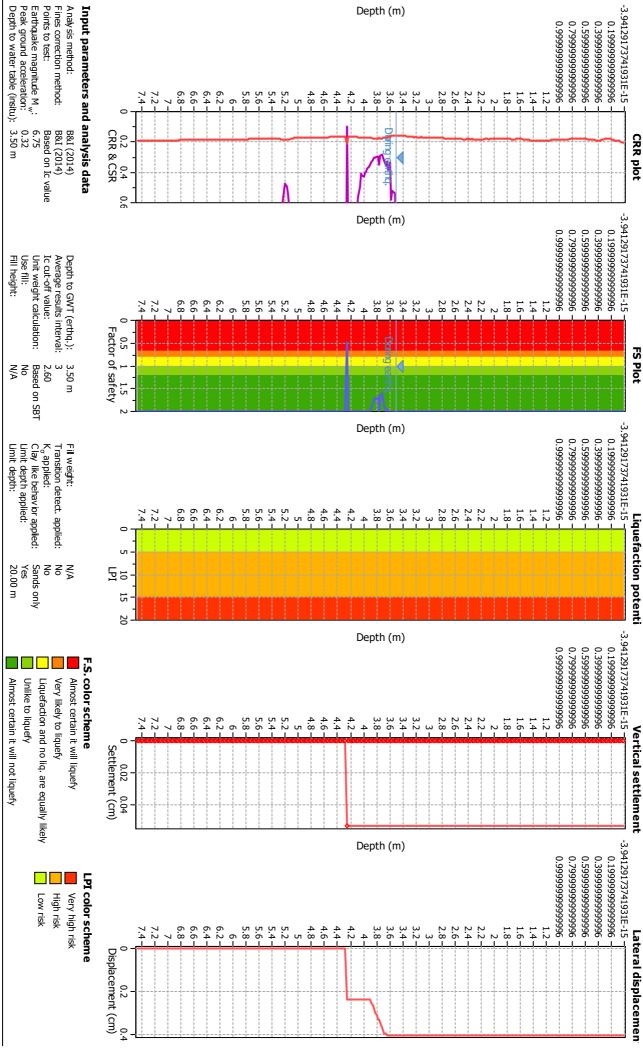
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Zone A₁: Cyclic liquefaction likely depending on size and duration of cyclic loading Zone A₂: Cyclic liquefaction and strength loss likely depending on loading and ground



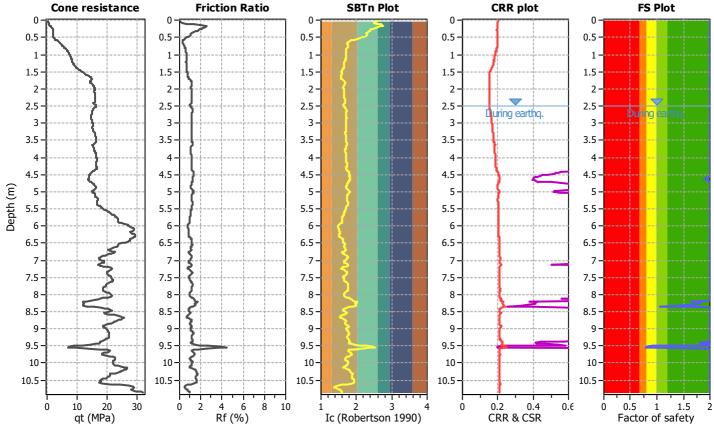


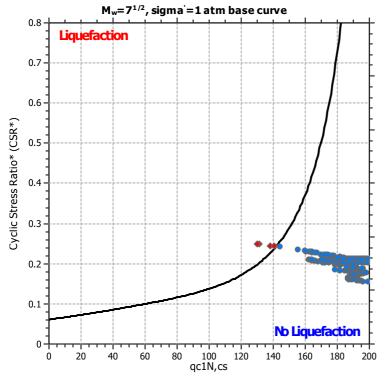
Project title: 195340402 Location: 131 Otaihanga Rd, Paraparaumu

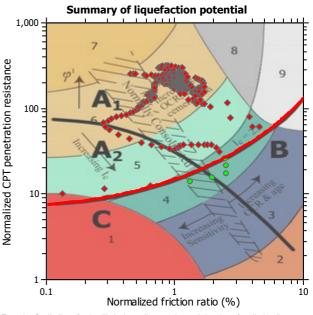
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Input parameters and analysis data

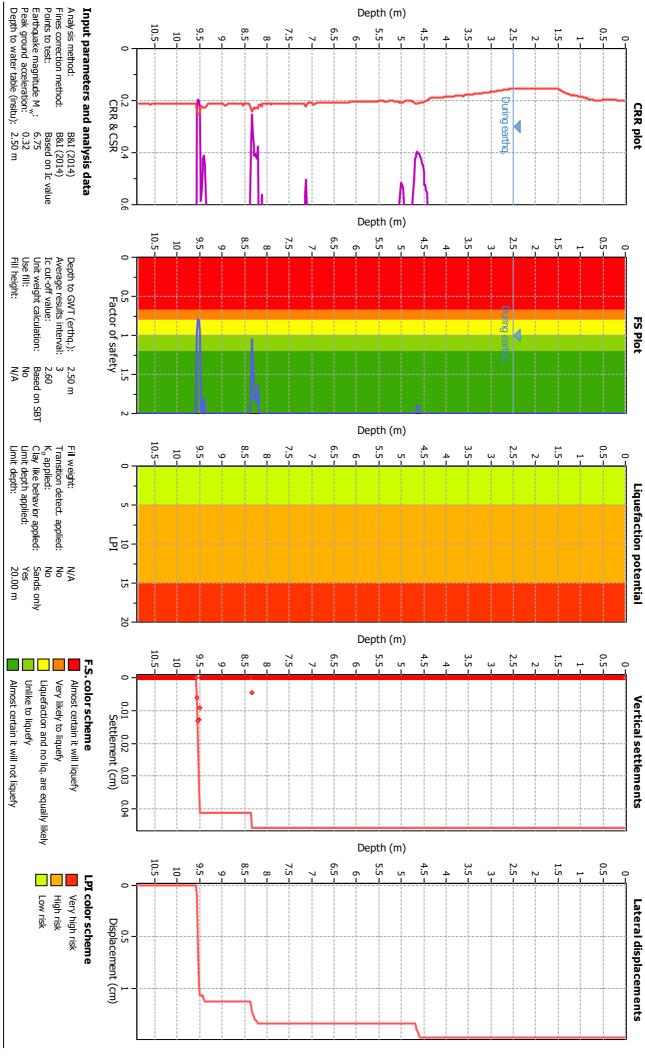
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Zone A₁: Cyclic liquefaction likely depending on size and duration of cyclic loading Zone A₂: Cyclic liquefaction and strength loss likely depending on loading and ground



CLiq v.3.0.2.4 - CPT Liquefaction Assessment Software - Report created on: 15/04/2020, 5:44:39 PM Project file: S:\Projects\195340402_131 Otaihanga Rd\CLiq\195340402_CLiq.clq

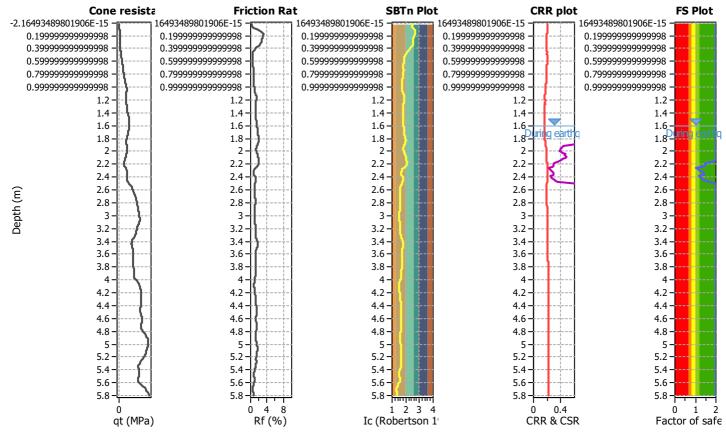


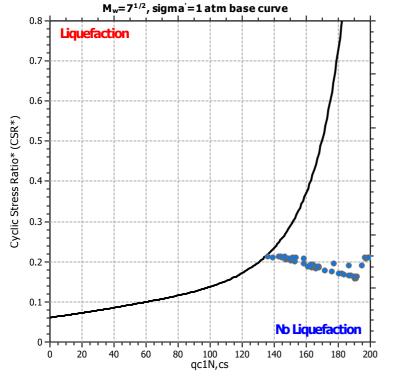
Project title: 195340402 Location: 131 Otaihanga Rd, Paraparaumu

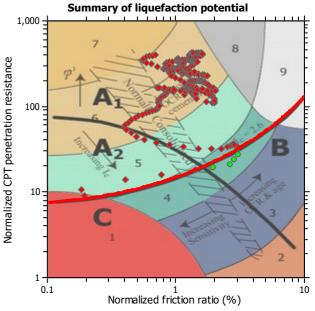
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Input parameters and analysis data

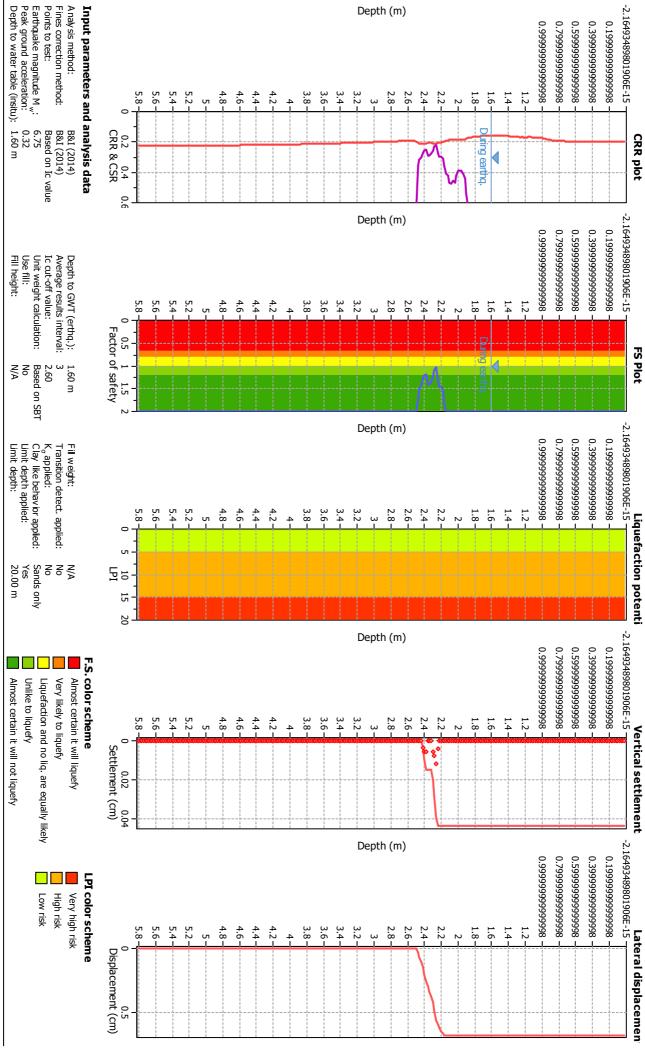
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Zone A₁: Cyclic liquefaction likely depending on size and duration of cyclic loading Zone A₂: Cyclic liquefaction and strength loss likely depending on loading and ground





Project title: 195340402 Location: 131 Otaihanga Rd, Paraparaumu

CPT file: CPT05_ULS

Peak ground acceleration:

Input parameters and analysis data

A naly sis method: B&I of Fines correction method: B&I of Points to test: Base Earthquake magnitude M w: 6.75

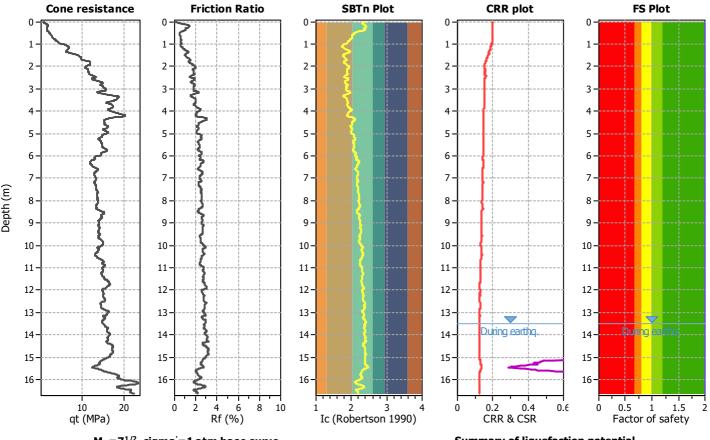
B&I (2014) B&I (2014) Based on Ic value 6.75 G.W.T. (in-situ): G.W.T. (earthq.): Average results interval: Ic cut-off value: Unit weight calculation:

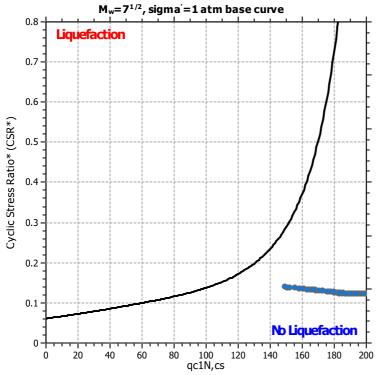
13.50 m 13.50 m 3 2.60 Based on SBT $\begin{array}{lll} \text{Use fill:} & \text{No} \\ \text{Fill height:} & \text{N/A} \\ \text{Fill weight:} & \text{N/A} \\ \text{Trans. detect. applied:} & \text{No} \\ \text{K}_{\sigma} \, \text{applied:} & \text{No} \end{array}$

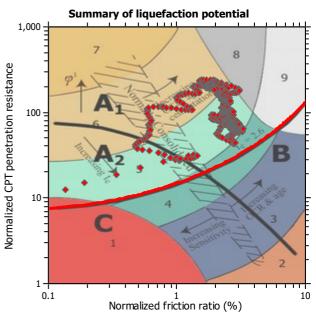
Clay like behavior applied: Limit depth applied: Limit depth:

MSF method:

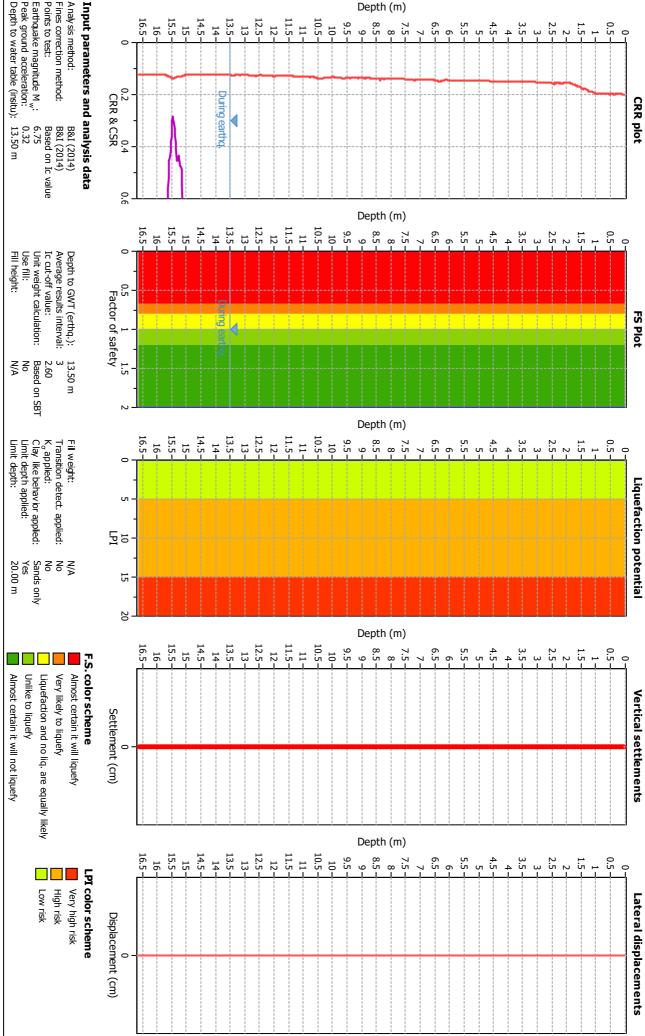
Sands only I: Yes 20.00 m Method based







Zone A_1 : Cyclic liquefaction likely depending on size and duration of cyclic loading Zone A_2 : Cyclic liquefaction and strength loss likely depending on loading and ground geometry



Depth (m)

Project file: S:\Projects\195340402_131 Otaihanga Rd_Mr Mansell\04 Assessment\131 Otaihanga Rd\CLiq\195340402_CLiq.dq CLiq v.3.0.2.4 - CPT Liquefaction Assessment Software - Report created on: 15/04/2020, 5:44:41 PM

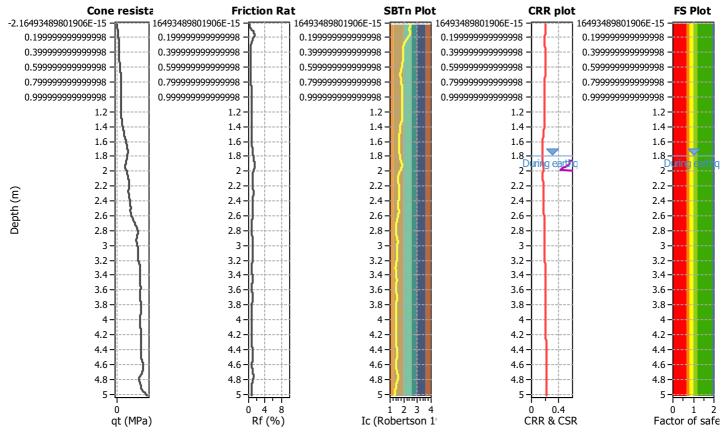


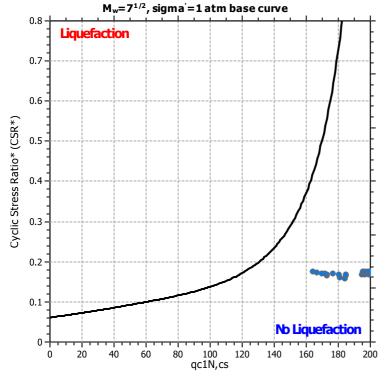
Project title: 195340402 Location: 131 Otaihanga Rd, Paraparaumu

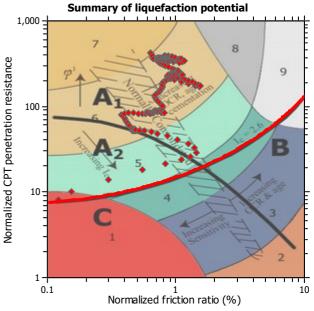
CPT file: CPT06_ULS

Input parameters and analysis data

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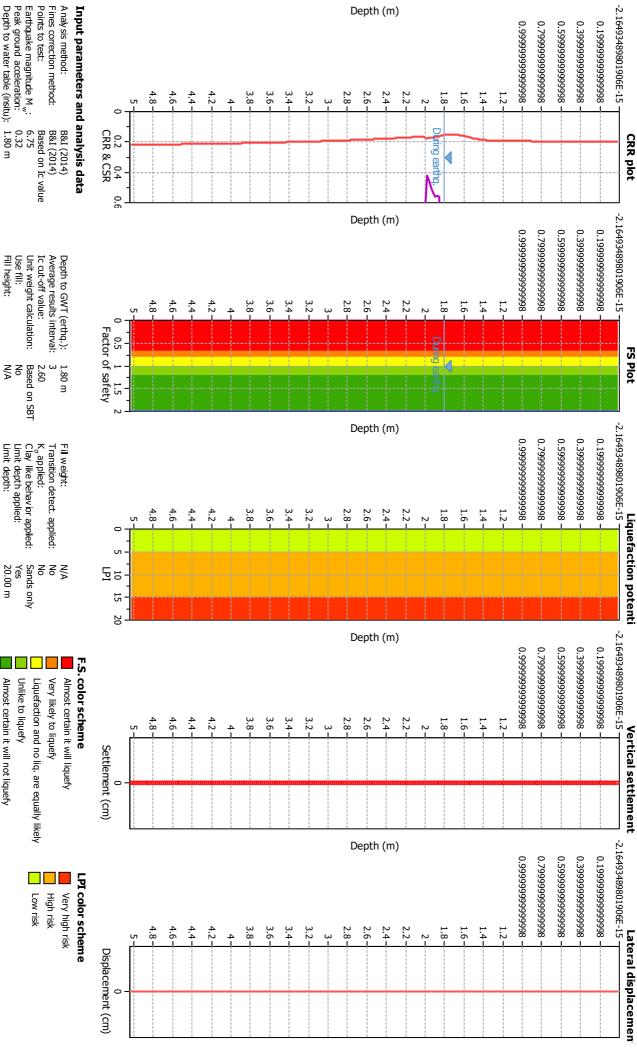






Zone A₁: Cyclic liquefaction likely depending on size and duration of cyclic loading Zone A₂: Cyclic liquefaction and strength loss likely depending on loading and ground







Project title: 195340402 Location: 131 Otaihanga Rd, Paraparaumu

CPT file: CPT07_ULS

Input parameters and analysis data

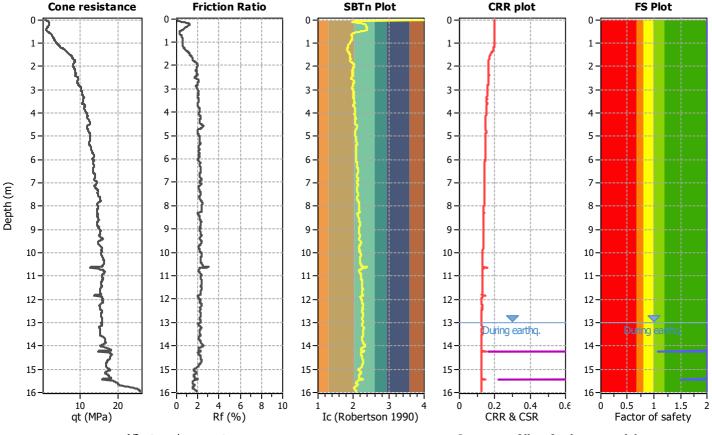
A naly sis method:
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Points to test:
Earthquake magnitude M w:
Peak ground acceleration:

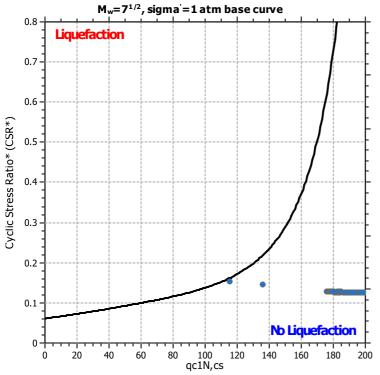
B&I (2014)
B&I (2014)
Based on Ic value
6.75
0.32

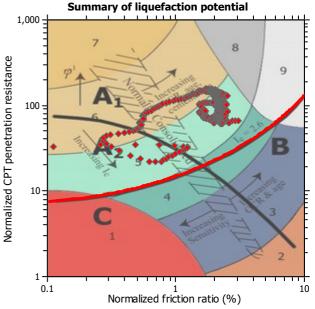
) G.W.T. (in-situ):
) G.W.T. (earthq.):
Ic value Average results interval:
Ic cut-off value:
Unit weight calculation:

13.00 m 13.00 m val: 3 2.60 n: Based on SBT $\begin{array}{lll} \text{Use fill:} & \text{No} \\ \text{Fill height:} & \text{N/A} \\ \text{Fill weight:} & \text{N/A} \\ \text{Trans. detect. applied:} & \text{No} \\ \text{K}_{\sigma} \, \text{applied:} & \text{No} \end{array}$

Clay like behavior applied: Sands only Limit depth applied: Yes Limit depth: 20.00 m MSF method: Method based







Zone A_1 : Cyclic liquefaction likely depending on size and duration of cyclic loading Zone A_2 : Cyclic liquefaction and strength loss likely depending on loading and ground geometry

Depth to water table (insitu): Earthquake magnitude M_w: A naly sis method: Input parameters and analysis data Peak ground acceleration: Points to test: Fines correction method: 14-14.5-13-13.5-9.5· 10· 10.5· 11· 11.5· 12.5· 8.5 7.5 5.5 4.5 3.5 2.5 1.5 0.2 0.4 CRR & CSR During earthc **CRR plot** 13.00 m 6.75 0.32 B&I (2014) B&I (2014) Based on Ic value 0.6 Depth (m) 5.5 6.5 7.5 7.5 8.5 8.5 9.5 10 10.5 11.5 13-13.5-12.5-14 45 3.5 2.5 1.5 4 Unit weight calculation: Use fill: Fill height: Average results interval: Depth to GWT (erthq.): Ic cut-off value: Factor of safety **FS Plot** Based on SBT No N/A 13.00 m 3 2.60 Depth (m) K_o applied: Clay like behav or applied: Limit depth applied: 10.5-14.5 13.5 12.5 11.5 7.5 8.5 8.5 9.5 14 5.5 6.5 7 . 5 3.5 3 Fill weight: 13 12 Η 2.5 1.5 Transition detect. applied: б 4 Liquefaction potential G N/A No No Sands only Yes 20.00 m 15 Depth (m) 11.5 11.5 12.5 12.5 13.5 13.5 5.5. 6.5. 7.5. 8.5. 8.5. 9.5. 10.5. 14.5 4.5 5. 1.5 3.5 2.5 4 ω 2 F.S. color scheme Almost certain it will liquefy Very likely to liquefy Liquefaction and no liq. are equally likely Almost certain it will not liquefy Unlike to liquery Vertical settlements 0.001 0.001 0.002 Settlement (cm) 0.002 Depth (m) 5.5. 6.5. 7.5. 8.5. 8.5. 9.5. 10.5. 14.5 13. 13.5 12.5 11.5 4.5 3.5 2.5 1.5 LPI color scheme High risk Very high risk Low risk **Lateral displacements** Displacement (cm) 0.02 0.04 0.06

Depth (m)

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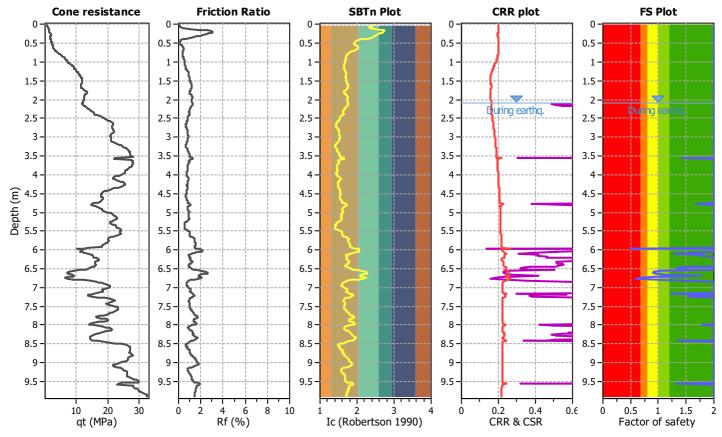


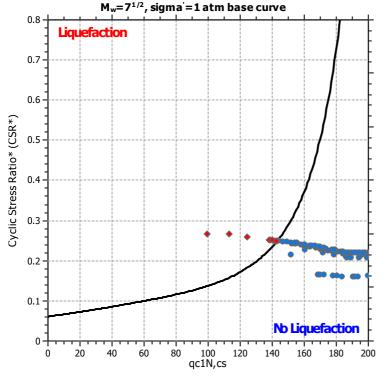
Project title: 195340402 Location: 131 Otaihanga Rd, Paraparaumu

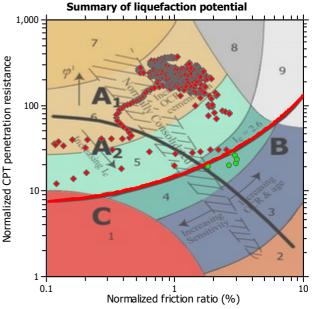
CPT file: CPT08_ULS

Input parameters and analysis data

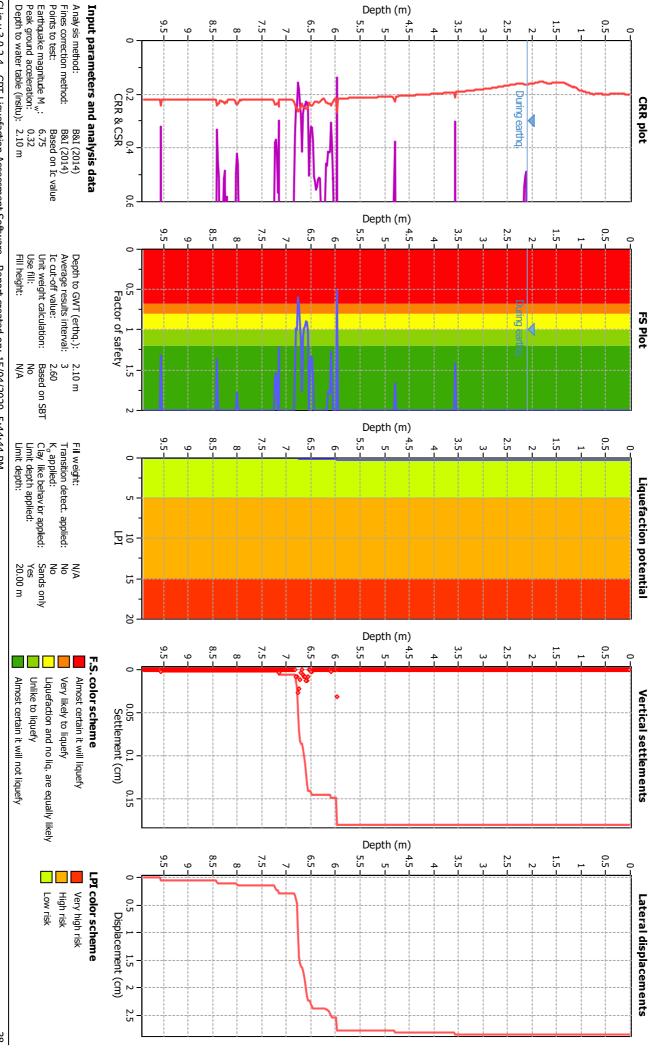
Clay like behavior A naly sis method: B&I (2014) G.W.T. (in-situ): 2.10 m Use fill: No G.W.T. (earthq.): Fines correction method: B&I (2014) 2.10 m Fill height: N/A applied: Sands only Points to test: Based on Ic value Average results interval: 3 Fill weight: N/A Limit depth applied: Yes Earthquake magnitude M w: 6.75 Ic cut-off value: 2.60 Trans. detect. applied: No Limit depth: 20.00 m Peak ground acceleration: Unit weight calculation: Based on SBT K_{σ} applied: No MSF method: Method based







Zone A_1 : Cyclic liquefaction likely depending on size and duration of cyclic loading Zone A_2 : Cyclic liquefaction and strength loss likely depending on loading and ground geometry



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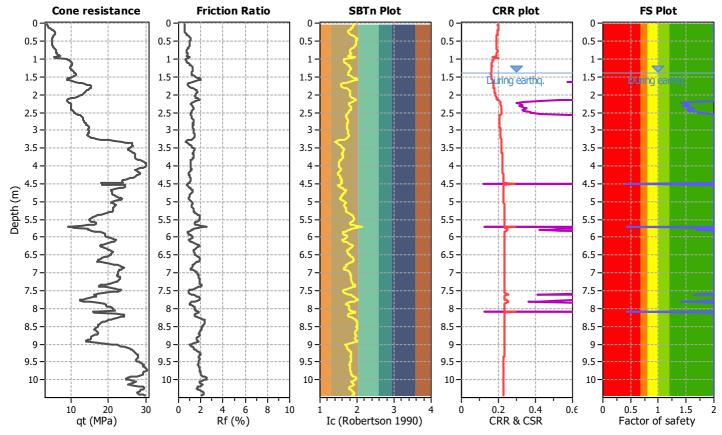


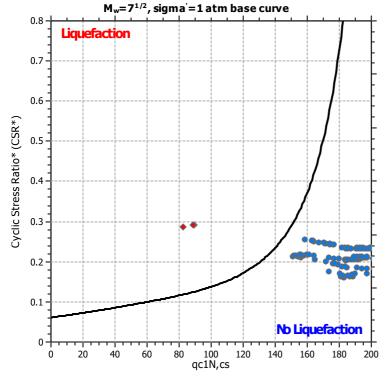
Project title: 195340402 Location: 131 Otaihanga Rd, Paraparaumu

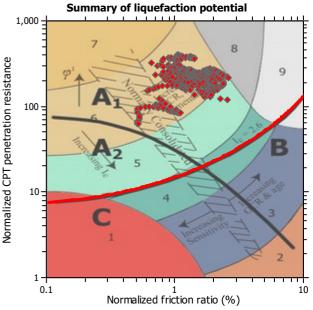
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Input parameters and analysis data

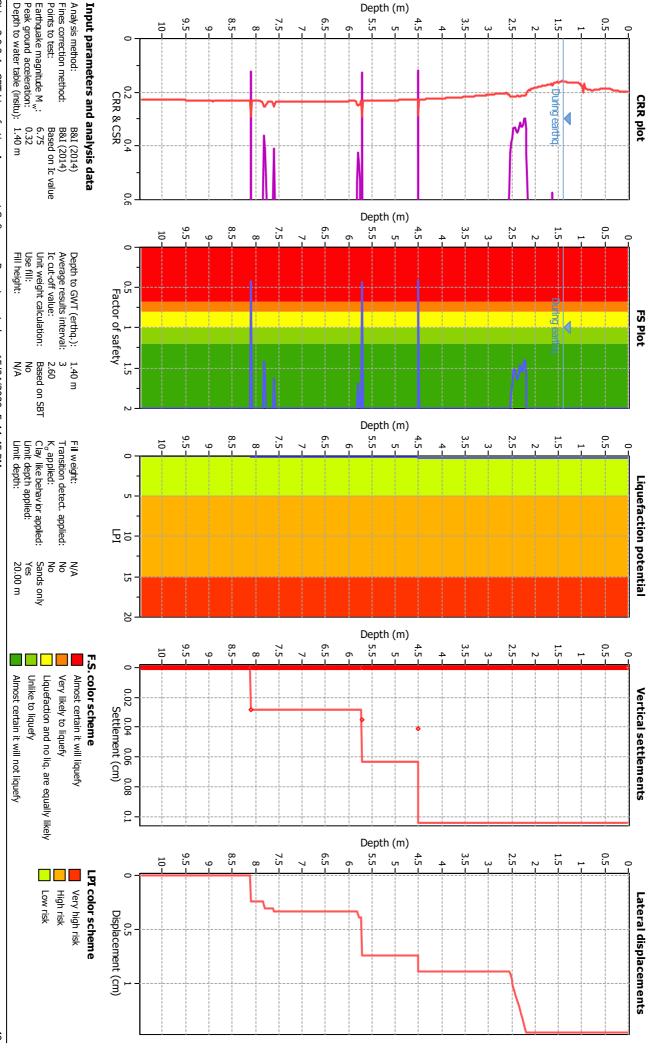
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Zone A₁: Cyclic liquefaction likely depending on size and duration of cyclic loading Zone A₂: Cyclic liquefaction and strength loss likely depending on loading and ground



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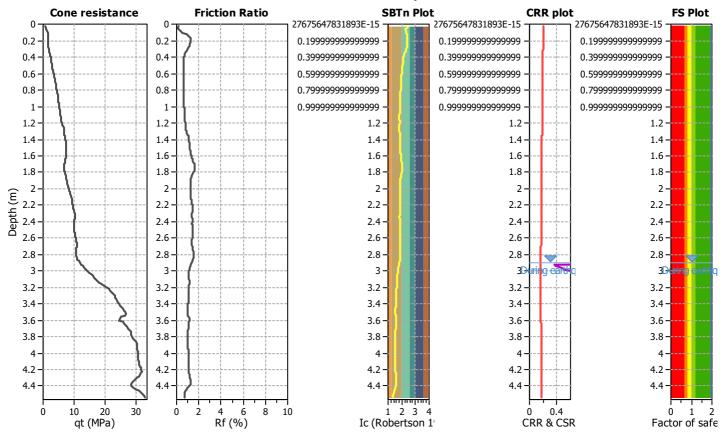


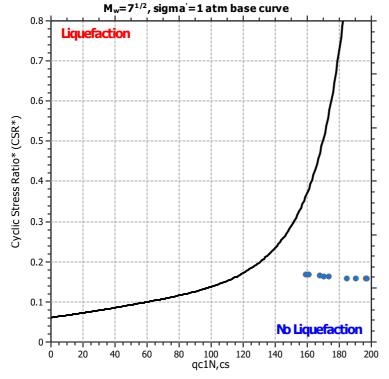
Project title: 195340402 Location: 131 Otaihanga Rd, Paraparaumu

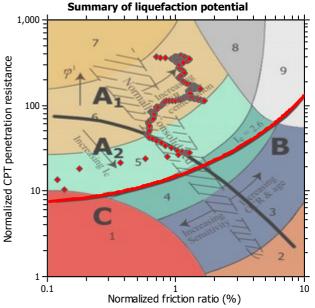
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Input parameters and analysis data

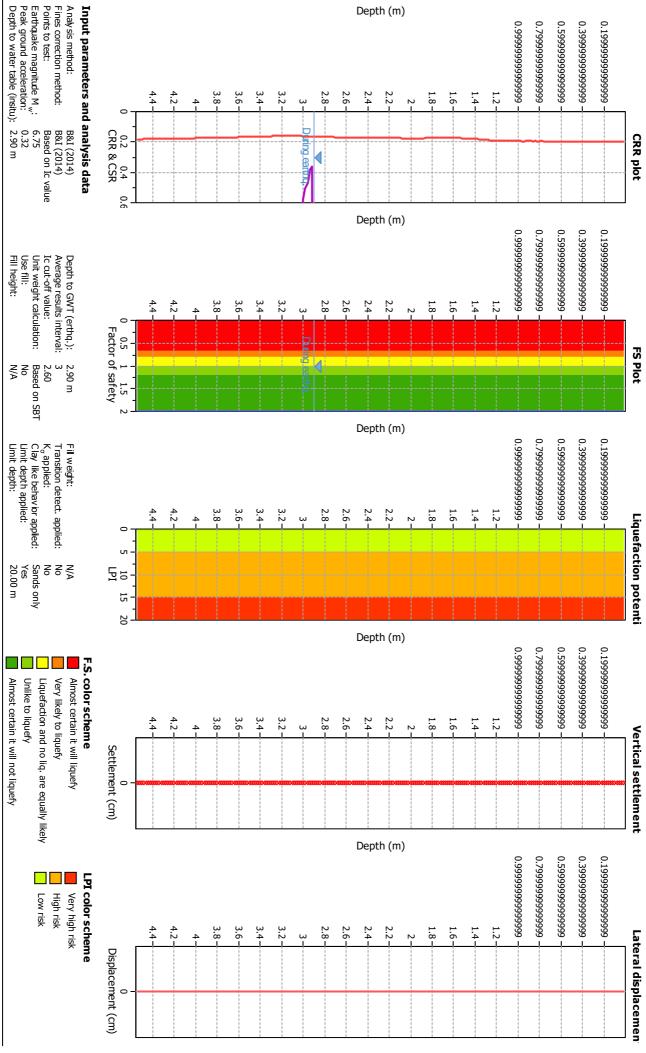
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Zone A_1 : Cyclic liquefaction likely depending on size and duration of cyclic loading Zone A_2 : Cyclic liquefaction and strength loss likely depending on loading and ground geometry



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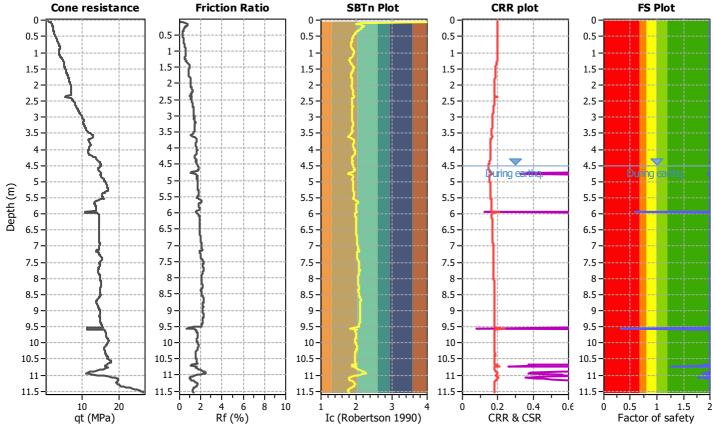


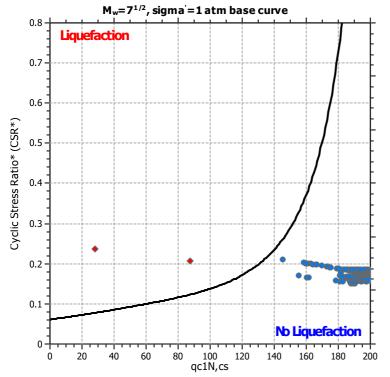
Project title: 195340402 Location: 131 Otaihanga Rd, Paraparaumu

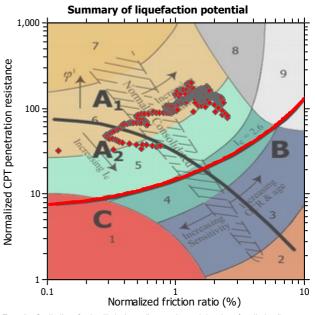
CPT file: CPT11_ULS

Input parameters and analysis data

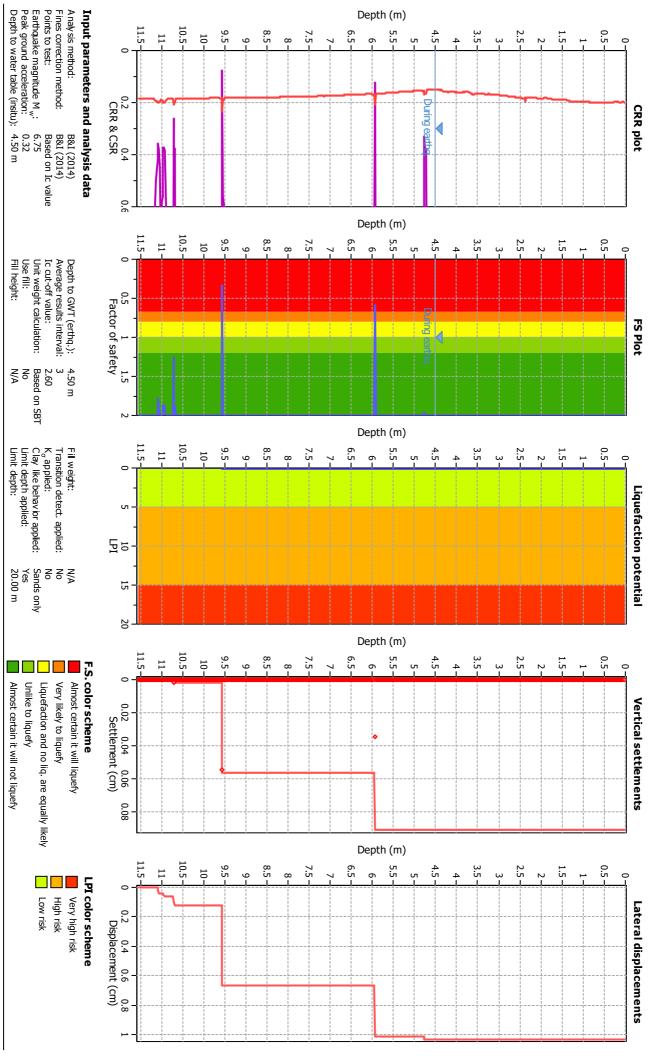
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Zone A_1 : Cyclic liquefaction likely depending on size and duration of cyclic loading Zone A_2 : Cyclic liquefaction and strength loss likely depending on loading and ground geometry



CLiq v.3.0.2.4 - CPT Liquefaction Assessment Software - Report created on: 15/04/2020, 5:44:47 PM Project file: S:\Projects\195340402_131 Otaihanga Rd\CLiq\195340402_CLiq.clq

Important Information about This

Geotechnical-Engineering Report

Subsurface problems are a principal cause of construction delays, cost overruns, claims, and disputes.

While you cannot eliminate all such risks, you can manage them. The following information is provided to help.

The Geoprofessional Business Association (GBA) has prepared this advisory to help you - assumedly a client representative - interpret and apply this geotechnical-engineering report as effectively as possible. In that way, clients can benefit from a lowered exposure to the subsurface problems that, for decades, have been a principal cause of construction delays, cost overruns, claims, and disputes. If you have questions or want more information about any of the issues discussed below, contact your GBA-member geotechnical engineer. **Active involvement in the Geoprofessional Business** Association exposes geotechnical engineers to a wide array of risk-confrontation techniques that can be of genuine benefit for everyone involved with a construction project.

Geotechnical-Engineering Services Are Performed for Specific Purposes, Persons, and Projects

Geotechnical engineers structure their services to meet the specific needs of their clients. A geotechnical-engineering study conducted for a given civil engineer will not likely meet the needs of a civil-works constructor or even a different civil engineer. Because each geotechnical-engineering study is unique, each geotechnical-engineering report is unique, prepared solely for the client. Those who rely on a geotechnical-engineering report prepared for a different client can be seriously misled. No one except authorized client representatives should rely on this geotechnical-engineering report without first conferring with the geotechnical engineer who prepared it. And no one – not even you – should apply this report for any purpose or project except the one originally contemplated.

Read this Report in Full

Costly problems have occurred because those relying on a geotechnicalengineering report did not read it *in its entirety*. Do not rely on an executive summary. Do not read selected elements only. *Read this report in full*.

You Need to Inform Your Geotechnical Engineer about Change

Your geotechnical engineer considered unique, project-specific factors when designing the study behind this report and developing the confirmation-dependent recommendations the report conveys. A few typical factors include:

- the client's goals, objectives, budget, schedule, and risk-management preferences;
- the general nature of the structure involved, its size, configuration, and performance criteria;
- · the structure's location and orientation on the site; and
- other planned or existing site improvements, such as retaining walls, access roads, parking lots, and underground utilities.

Typical changes that could erode the reliability of this report include those that affect:

- the site's size or shape;
- the function of the proposed structure, as when it's changed from a parking garage to an office building, or from a light-industrial plant to a refrigerated warehouse;
- the elevation, configuration, location, orientation, or weight of the proposed structure;
- the composition of the design team; or
- · project ownership.

As a general rule, *always* inform your geotechnical engineer of project changes – even minor ones – and request an assessment of their impact. The geotechnical engineer who prepared this report cannot accept responsibility or liability for problems that arise because the geotechnical engineer was not informed about developments the engineer otherwise would have considered.

This Report May Not Be Reliable

Do not rely on this report if your geotechnical engineer prepared it:

- for a different client;
- for a different project;
- for a different site (that may or may not include all or a portion of the original site); or
- before important events occurred at the site or adjacent to it; e.g., man-made events like construction or environmental remediation, or natural events like floods, droughts, earthquakes, or groundwater fluctuations.

Note, too, that it could be unwise to rely on a geotechnical-engineering report whose reliability may have been affected by the passage of time, because of factors like changed subsurface conditions; new or modified codes, standards, or regulations; or new techniques or tools. If your geotechnical engineer has not indicated an "apply-by" date on the report, ask what it should be, and, in general, if you are the least bit uncertain about the continued reliability of this report, contact your geotechnical engineer before applying it. A minor amount of additional testing or analysis – if any is required at all – could prevent major problems.

Most of the "Findings" Related in This Report Are Professional Opinions

Before construction begins, geotechnical engineers explore a site's subsurface through various sampling and testing procedures. *Geotechnical engineers can observe actual subsurface conditions only at those specific locations where sampling and testing were performed.* The data derived from that sampling and testing were reviewed by your geotechnical engineer, who then applied professional judgment to form opinions about subsurface conditions throughout the site. Actual sitewide-subsurface conditions may differ – maybe significantly – from those indicated in this report. Confront that risk by retaining your geotechnical engineer to serve on the design team from project start to project finish, so the individual can provide informed guidance quickly, whenever needed.

This Report's Recommendations Are Confirmation-Dependent

The recommendations included in this report – including any options or alternatives – are confirmation-dependent. In other words, they are not final, because the geotechnical engineer who developed them relied heavily on judgment and opinion to do so. Your geotechnical engineer can finalize the recommendations only after observing actual subsurface conditions revealed during construction. If through observation your geotechnical engineer confirms that the conditions assumed to exist actually do exist, the recommendations can be relied upon, assuming no other changes have occurred. The geotechnical engineer who prepared this report cannot assume responsibility or liability for confirmation-dependent recommendations if you fail to retain that engineer to perform construction observation.

This Report Could Be Misinterpreted

Other design professionals' misinterpretation of geotechnicalengineering reports has resulted in costly problems. Confront that risk by having your geotechnical engineer serve as a full-time member of the design team, to:

- confer with other design-team members,
- help develop specifications,
- review pertinent elements of other design professionals' plans and specifications, and
- be on hand quickly whenever geotechnical-engineering guidance is needed.

You should also confront the risk of constructors misinterpreting this report. Do so by retaining your geotechnical engineer to participate in prebid and preconstruction conferences and to perform construction observation.

Give Constructors a Complete Report and Guidance

Some owners and design professionals mistakenly believe they can shift unanticipated-subsurface-conditions liability to constructors by limiting the information they provide for bid preparation. To help prevent the costly, contentious problems this practice has caused, include the complete geotechnical-engineering report, along with any attachments or appendices, with your contract documents, but be certain to note conspicuously that you've included the material for informational purposes only. To avoid misunderstanding, you may also want to note that "informational purposes" means constructors have no right to rely on the interpretations, opinions, conclusions, or recommendations in the report, but they may rely on the factual data relative to the specific times, locations, and depths/elevations referenced. Be certain that constructors know they may learn about specific project requirements, including options selected from the report, only from the design drawings and specifications. Remind constructors that they may

perform their own studies if they want to, and *be sure to allow enough time* to permit them to do so. Only then might you be in a position to give constructors the information available to you, while requiring them to at least share some of the financial responsibilities stemming from unanticipated conditions. Conducting prebid and preconstruction conferences can also be valuable in this respect.

Read Responsibility Provisions Closely

Some client representatives, design professionals, and constructors do not realize that geotechnical engineering is far less exact than other engineering disciplines. That lack of understanding has nurtured unrealistic expectations that have resulted in disappointments, delays, cost overruns, claims, and disputes. To confront that risk, geotechnical engineers commonly include explanatory provisions in their reports. Sometimes labeled "limitations," many of these provisions indicate where geotechnical engineers' responsibilities begin and end, to help others recognize their own responsibilities and risks. *Read these provisions closely*. Ask questions. Your geotechnical engineer should respond fully and frankly.

Geoenvironmental Concerns Are Not Covered

The personnel, equipment, and techniques used to perform an environmental study – e.g., a "phase-one" or "phase-two" environmental site assessment – differ significantly from those used to perform a geotechnical-engineering study. For that reason, a geotechnical-engineering report does not usually relate any environmental findings, conclusions, or recommendations; e.g., about the likelihood of encountering underground storage tanks or regulated contaminants. Unanticipated subsurface environmental problems have led to project failures. If you have not yet obtained your own environmental information, ask your geotechnical consultant for risk-management guidance. As a general rule, do not rely on an environmental report prepared for a different client, site, or project, or that is more than six months old.

Obtain Professional Assistance to Deal with Moisture Infiltration and Mold

While your geotechnical engineer may have addressed groundwater, water infiltration, or similar issues in this report, none of the engineer's services were designed, conducted, or intended to prevent uncontrolled migration of moisture – including water vapor – from the soil through building slabs and walls and into the building interior, where it can cause mold growth and material-performance deficiencies. Accordingly, proper implementation of the geotechnical engineer's recommendations will not of itself be sufficient to prevent moisture infiltration. Confront the risk of moisture infiltration by including building-envelope or mold specialists on the design team. Geotechnical engineers are not building-envelope or mold specialists.



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