



Appendix G

Conceptual Stormwater Disposal Design Report

Prepared by Cuttriss Consultants Ltd



Conceptual Stormwater Disposal Design Report for Resource Consent Application

Lot 12 DP 90944, 160 Mazengarb Road, Paraparaumu

Ref: 23333

29 July 2024

Prepared for: Sussex Trust
160 Mazengarb Road, Paraparaumu

CONCEPTUAL STORMWATER DISPOSAL DESIGN FOR PROPOSED RESIDENTIAL DEVELOPMENT AT LOT 12 DP 90944, 160 MAZENGARB ROAD, PARAPARAUMU

Following the engagement of our services for investigating and reporting on stormwater disposal for resource consent application at the above site, we have excavated one test pit, and a soakage test was carried out within the pit. The test has been undertaken to investigate the soakage characteristics of the underlying material. We detail our findings and conceptual design approach for stormwater disposal below.

1. PREAMBLE

This report has been prepared to provide a conceptual stormwater disposal design for the proposed development on this site including impervious areas associated with the dwelling roofs, driveways, paving, roading, and footpaths within the development.

The site is zoned general residential and is surrounded by residential properties. The site has one existing dwelling located towards the rear of the property. There are also multiple other outbuildings such as a shed, garage and barn. Towards the front of the property is an area that has been utilised as a paddock for grazing horses.

The proposed development proposes 41 standalone residential lots with the development site area encompassing 7,168m². The development proposal is shown on the plans included within Appendix A.

The report has been prepared to provide stormwater disposal options and to confirm hydraulic neutrality in accordance with the infrastructure policy on managing the effects on network utilities as detailed in Part 2 of the *Kapiti Coast District Council Operative District Plan – rule INF-MENU-P17 Hydraulic Neutrality - Stormwater*.

Our conceptual design intent for managing stormwater of the proposed development is as follows:

- The development proposes a centralised stormwater disposal system. A crate modular type system will allow soakage and accommodate storage for development runoff up to a 1% AEP event.
- Residential Development – runoff from the individual residential lot areas will be captured by direct connections to stormwater mains and street catchpits (via kerb adaptors) and will be conveyed via the piped stormwater network. An allowance has been made for pervious and impervious areas to be captured and conveyed to the centralised stormwater disposal system.

Note: Roof water runoff from lots 20 – 39 will connect to a shared stormwater line those feeds into the proposed onsite 20,000 litre water re-use tank. Overflow from this tank will enter the centralised stormwater disposal area via an overflow pipe. Location and levels of the overflow pipe will require careful consideration at detailed design stage.

- Residential Road Reserve – runoff captured by street catchpits will be directed by a piped network allowing for a 1% AEP. An allowance has been made for pervious and impervious areas to be captured and conveyed to the centralised stormwater disposal system.

Secondary overflow will be provided from the soakage modules via a connection to an existing stormwater pipe along the southern boundary. The Mannings formula was used to calculate the flow capacity of the existing 150mm connector pipe, with known levels taken from KCDC GIS. The 150mm pipe was confirmed as being able to convey the flow difference between the 1% AEP event and the 0.5% AEP event.

The basis of the design is NZBC E1 from the approved document prepared by the Ministry of Building, Innovation and Employment (MBIE). NZBC E1 details the rainfall intensity used in the design as being an event having a 1 hour duration and a 10% probability of occurring annually (1 in 10-year event). However, a 1% AEP event (1 in 100 year event) has been considered in this design in accordance with KCDC requirements.

A 10% AEP and 1% AEP 24hr duration nested storm event was used to calculate the runoff volume generated from each catchment using HEC-HMS. HEC-HMS has been used to model the required attenuation volume for a 1% AEP event. Storm intensities were derived from HIRDS V4.

2. DOCUMENTS

The following documents are referred to within this report:

- Development proposal (Appendix A)
- General and Site Testing Photos (Appendix B)
- Design Drawings (Appendix C)
- Calculations (Appendix D)
- Designgroup Stapleton Elliot - Architect (Appendix E)
- Torlesse Ltd – Geotechnical Engineer (Appendix F)
- KCDC Land Development Minimum Requirements 2022 (KCDC LDMR)
- New Zealand Building Code E1: Surface Water (NZBC E1)
- KCDC Operative District Plan 2021
- WWL Reference Guide for Design Storm Hydrology April 2019

Rainfall data for determining the anticipated stormwater rainfall intensity has been sourced from the NIWA High Intensity Rainfall Design System V4 (HIRDS V4) RCP6 to 2100 has been

factored in accordance with the WWL Reference Guide for Design Storm Hydrology April 2019 documentation.

3. LOCATION

The site is located off Mazengarb Road and is reasonably well defined by way of existing fence along all boundaries. Other residual dwellings are located on the adjoining boundaries. The site is located within proximity to Paraparaumu College (approximately 90m), Mazengarb Park (approximately 280m) and the Kapiti Expressway (approximately 1.1km).

The location of the subject site is fully detailed within the resource consent application documentation.

4. TOPOGRAPHY

The topography of the site consists gently undulating landforms that range from approximately RL 7.1 to RL 4.1.

The southeast and southwest (rear) boundaries of the property have timber retaining walls with fencing on top as the ground level is lower than the surrounding properties at these points. The northwest and northeast (front) boundaries are fenced with no retaining walls.

The topography is shown with existing contours within the scheme plans (23333 SCH sheet 1 of 19).

The site conditions and general topography will be suitable for on-site stormwater disposal, with consideration being required at detailed design stage with respect to stormwater sump lead pipe grades (from road carriageway areas) and pipe cover leading to the soakage areas.

Soil conditions generally comprise of underlying sands. These conditions have been confirmed by way of excavated test pit during the testing process to confirm disposal options for stormwater generated from the development.

5. TESTING PROCEDURE

One test pit was excavated on site as part of the investigation carried out with the soakage test location shown on the appended plan. The test pit was excavated by Piling and Drilling Ltd on 8 July 2024. The locations of the test pits are included within Appendix B.

Test pit A measured 1.6 metres (wide) x 1.2 metres (long) at the top of the excavated pit, with the depth of excavation being 0.6 metres.

The water table was not encountered, during the excavation.

A vehicle with a mounted tank with 1,000 litres of water was used to discharge water to test pit A. Water was discharged to the excavated test pit by way of hose connected to a pump,

connected to the water tank. Soakage was also occurring while the test pit was filling up simultaneously.

At test location A after 38 minutes of water being discharged at a controlled low flow rate the pit was filled to 0.35 metres deep. The appended photos show the testing procedure.

The following information was recorded during the testing process

- Length of time to fill the test pit.
- The water level in the test pit while the water was being discharged into the test pit and the level once filling had stopped.

6. FACTOR OF SAFETY

Although not a requirement of the compliance document for the New Zealand Building Code (E1 Surface Water) it is recommended that the design soakage rates have an appropriate factor of safety applied to the raw soakage rates recorded.

A factor of safety of 4 is required by KCDC as being an acceptable factor of safety for soakpit designs in the Kapiti Coast district (refer KCDC Subdivision and Development Principals and Requirements 2012 document).

7. RESULTS

The results of the testing are summarised below and appended to this report in a table and graphical format. We note on 2 July there was 30.8mm of rainfall and 3 July 3.8mm of rainfall but after this there was no more rainfall preceding the testing on the site¹.

For test pit A -

- 1,000 litres of water was emptied into the test pit. The fill took 38 minutes at this time both soaking and filling were occurring.
- The water level in the test pit was recorded at 0.3 metres below existing ground level after 38 minutes of water being discharged into the test pit - i.e. 0.35 metres of water in test pit
- The average rate of fall of water in the test pit between the period 10:31hrs – 11:10hrs the raw soakage rate of 540mm/hr

The results are summarised in Table 1 below and appended to this report in a table format.

¹ Source – GWRC rainfall data (Waikanae River at Water Treatment Plant) for period 1st July to 8th July 2024

Table 1 Soakage test results

| Test no. | Predominant soil | Location | Raw soakage (mm/hr) | Design soakage ² (mm/hr) |
|----------|------------------|-------------------|---------------------|-------------------------------------|
| A | sand | as shown on plans | 540 | 135 |

The soakage rate indicates that typical low impact urban designs (e.g. soakpits or soak trenches) are suitable for this site.

8. GROUND WATER

Ground water was not encountered during site testing however the depth of the excavated pit for testing was only 0.6m.

At the time of testing Torlesse Ltd were onsite undertaking geotechnical investigation involving scala penetrometer, CPT and test pit excavations. Reference is made to their report dated 24 July 2024 (Appendix F) which details investigation records. Three test locations are referenced:

- Test pit 03 (TP03) was undertaken relative to the proposed location of the stormwater disposal system – at approximately 2.5m below ground level (RL5.0m) moist to wet ground conditions are noted.
- Test pit 05 (TP05) was undertaken at the lowest point of the site – at approximately 1.3m below ground level (RL4.4m) moist ground conditions are noted.
- Test pit 06 (TP06) was undertaken relative to the proposed location of the stormwater disposal system – at approximately 3.0m below ground level (RL4.5m) wet ground conditions are noted.

Torlesse conclude in their report that an estimate of the groundwater level is around RL2.0m, being 2.5m to 4.0m below ground level.

9. CONCEPTUAL DESIGN

The conceptual design intent is outlined in the Preamble, this being separate consideration of the following sub-catchments.

- dwelling roofs
- driveways
- paving

² Design soakage has a factor of safety of 4 applied for this particular site for the design for the Q100 event.

- roading
- footpaths

The highpoints of the development will be at the boundaries with the finished surface levels falling internally. All roading and runoff from other areas will also fall towards a centralised disposal area.

The proposed stormwater disposal system location is under the communal area next to the carparking. The proposed location is shown on the scheme plans Appendix C.

The following coefficients have been used in determining the likely runoff from this development – this has been referenced from NZBC E1 Table 1.

- Asphalt and concrete paved surfaces – 0.85
- Fully roofed and/or sealed developments – 0.90
- Lawn and berm area – 0.25

The assumed catchment delineation is outlined in Table 2 below:

Table 2 Catchment delineation

| Catchment Surface | Catchment Area (m ²) | Runoff Coefficient |
|-----------------------------|----------------------------------|--------------------|
| Roof | 2448 | 0.9 |
| Paving | 1646 | 0.85 |
| Footpath | 311 | 0.85 |
| Road | 1114 | 0.85 |
| Carpark | 809 | 0.85 |
| Refuse | 25 | 0.85 |
| Impermeable around building | 511 | 0.85 |
| Lawn | 306 | 0.25 |

9.1 Nested Storm Event – Pre and post development run-offs

A 10% AEP and 1% AEP 24hr duration nested storm event was used to calculate the runoff volume generated from each catchment. Storm intensities were derived from HIRDS V4.

HEC-HMS has been used to model the required attenuation volume, based on the proposed design soakage rate for a 10% and 1% AEP event.

The runoff that flows into the stormwater disposal system is calculated by applying the design hydrograph in 5-minute time steps alongside the flow leaving the system due to site soakage and the resulting cumulative storage required.

The design soakage input to the HEC model (0.015m³/s) is based on a soakage area of 400m² being provided with a soakage rate of 135mm/hr.

The results from the HEC model are summarised below:

- In a 10% AEP event, allowing for soakage over a 24-hour event of 0.015m³/s. A storage requirement volume of **193m³** is required.
- In a 1% AEP event, allowing for soakage over a 24-hour event of 0.015m³/s. A storage requirement volume of **375m³** is required.

Further calculations and HEC-HMS model run outputs are available in Appendix D.

9.2 Stormwater Disposal System (Primary)

The proposed stormwater disposal system has been designed to cater for a 1% AEP event while providing soakage out the base of the crate system, storage is also considered to cater for flows greater than the design soakage rate.

A conceptual stormwater disposal design considering the Cirtex RainSmart interlocking module system is summarised below.

Conceptual Soakage System Dimensions:

Cirtex Soakage module

Length – 26m (36 units long @ 0.715m)

Width – 18m (40 units wide @ 0.4m)

Height – 0.86m (2 units @ 0.44m)

The above dimensions allow for storage volume of **378m³** based on a Cirtex void ratio of 0.95.

Careful consideration will need to be given at detailed design stage in relation to kerb levels, sump locations and levels, and associated sump leads heading to storage cells.

An inspection must be carried out on the base of the excavation at the time of construction, to confirm underlying sand material, for both options above.

9.3 Secondary Overflow (Secondary)

This report assesses how overland flow paths will be provided within the development to meet local LDMR requirements. The post development levels have been designed in such a way that the development is less 'intrusive' on the neighbouring properties e.g. not building up the site and having new dwellings overlooking properties. This has meant the site falls into a centralised stormwater disposal area and secondary overflow out to Mazengarb Road is not achievable.

Further considerations are required to determine other options available for secondary overflow from site in the event the proposed primary stormwater disposal system fails or is inundated with an event greater than a 1% AEP.

In an event that exceeds a 1% AEP event consideration has been given to connecting to the existing KCDC stormwater pipe along the southern boundary of the development, which is located on neighbouring properties 6, 12, 14 and 16 Niu Sila Way. The 300mm uPVC stormwater line (KCDC asset ID KSWP011663) has 3 existing 150mm connector pipes through an existing retaining wall for the development site.



Figure 1 – KCDC GIS data base existing services – Stormwater line along southern boundary

A stormwater pipe will be installed from the proposed stormwater disposal system to the southern most existing 150mm connector pipe (KCDC ID: KWSN016274). The flow capacity of the existing pipe has been undertaken using the Manning's formula which determined the pipe could convey **0.049m³/s**, having a velocity of **2.76m/s** based on a known grade of 1 in 150.



Figure 2 – KCDC GIS data base existing services – Connector Pipe (KSWWN016274)

As noted in the previous section 9.1 HEC-HMS has been used to model the required attenuation volume, based on the proposed design soakage rate for a 10% and 1% AEP event. The same HEC input file has been used to assess the attenuation volume difference between a 1% AEP event and a 0.5% AEP event. Further calculations and HEC-HMS model run outputs are available in Appendix D.

The calculation of the post-development attenuation volume difference for a 1% AEP and 0.5% AEP event can be considered as **96m³** having a peak discharge of **0.025m³/s** (1% AEP peak inflow less the 0.5% AEP peak inflow).

In summary the 150mm connector pipe could convey the difference of the 1% AEP event and 0.5% AEP in the event the site is subjected to a rain event greater than a 1% AEP event.

10. MAINTENANCE

As with any on site stormwater disposal system ongoing maintenance of the installed disposal system is the key to its effectiveness. Typically, our recommendations for the monitoring and maintenance of on-site stormwater disposal soakpits, which should be considered in the design, construction, and post-development phases, are as follows:

- *The soakage system needs to be checked weekly by the contractor during any subsequent construction works (after the soakage system is constructed), or after intense sediment deposit in the catchment area. On completion of all construction works the likelihood of sediment build-up will be reduced.*
- *Filter cloth should be applied over sumps to reduce the amount of sediment that could*

enter the soakage system during construction. Note: this must be removed following completion of construction.

- *The soakage chambers need to be checked by the asset owner every 3 months to monitor the amount of sediment build-up. If checks confirm sediment build-up is present then the system must be cleaned.*
- *The soakage chambers shall also be checked after intense rainfall events, and cleaned if sediment is discovered.*
- *Road sumps discharging to the soakage chamber need to be cleaned out by an appropriate sucker truck every 3 months. Sumps should also have appropriate filtering mechanisms installed.*
- *A detailed record should be kept by the asset owner detailing the dates of inspections undertaken, dates when sumps have been cleaned out and when sediment has been removed from the soakage systems.*
- *Refer to the manufacturer's requirements and maintenance procedures of the selected supplier.*

An operation and maintenance manual should be made available to the asset owner once the disposal systems have been constructed to enable them to plan routine maintenance for their asset. It is anticipated that an operation and maintenance manual will be required by a condition of consent.

11. FLOOD HAZARD ASSESSMENT – PONDING

The development site is subject to KCDC flood hazard overlay – Ponding. Information was sought from KCDC on the latest flood hazard detail as the current information available online (KCDC GIS) is outdated. Information was provided by KCDC to Cuttriss on the 02 May 2024 which identified isolated ponding across the site.

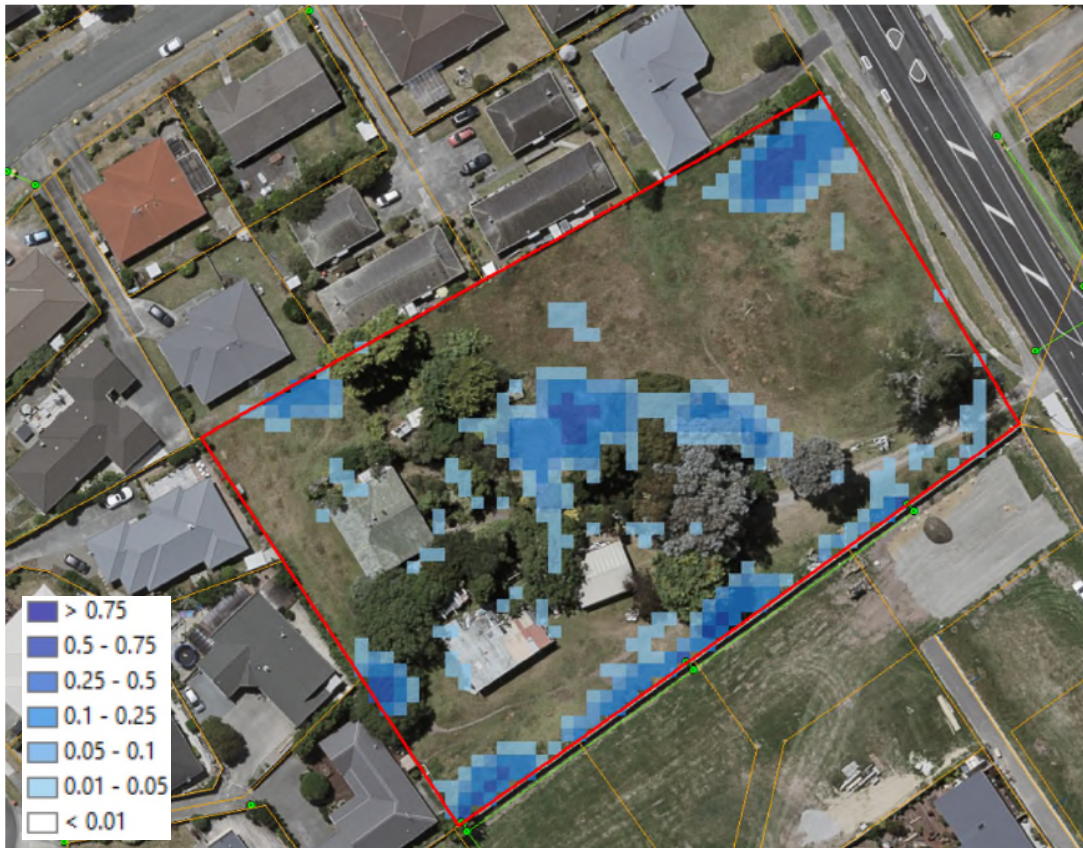


Figure 3 – KCDC Flood hazard – Ponding (02 May 2024)

Following a review of the flood information provided it can be concluded that the ponding occurring onsite is due to the site itself and not any external factors, with ponding shown in the low-lying areas of the site between 0.010 – 0.400m.

The proposed development levels will mitigate the risk of ponding based on the current identified hazard, which recommended that any building level (RBL) be 0.3 – 0.5m above top water levels to underside of floor or joist.

All stormwater created from the development of the site will be contained onsite as detailed in section 9? of this report.

12. CONCLUSION

This report has been prepared to demonstrate design options available for stormwater disposal from the proposed development. The results confirm that typical low impact urban designs (e.g. soakpits, soak trenches, or depression areas) would be suitable for this site. This is based on the observations of the materials present in the test pit excavated, and the volume of water disposed of into the excavated pit the site.

The primary stormwater disposal system incorporates a piped network capturing the 1% AEP event, which then discharges to a centralised stormwater disposal area designed to store up to a 1% AEP event while allowing soakage out the base.

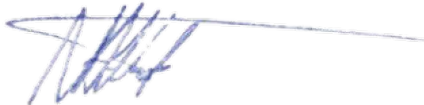
Secondary overflow has been considered by proposing a connection to the existing 150mm connector pipe in the southern end of the site. Calculations were undertaken using the Manning's formula to confirm the flow capacity of the pipe which is able to take the difference between the 1% AEP event and 0.5% AEP event.

Careful consideration will need to be made to the finished road and right of way levels and sump locations at the detailed design stage to allow runoff into the piped network.

An inspection must be carried out on the base of the excavation at the time of construction, to confirm underlying sand material, for both options above.

This report also considers the effects of the ponding flood hazard layer identified by KCDC on site. The proposed development levels will mitigate the risk of ponding based on the current identified hazard will all stormwater created from the development maintained onsite.

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APPENDIX A

Development Proposal

APPENDIX B

Photos of general site and
soakage testing undertaken



Above: 8 July 2024 – Underlying sand material in test pit



Above: 8 July 2024 – Test pit during testing



Above: 8 July 2024 – Looking southeast with test pit shown



Above: 8 July 2024 – Looking south with test pit shown



Above: 8 July 2024 – Looking southwest with test pit shown



Above: 8 July 2024 – Looking southwest with test pit shown

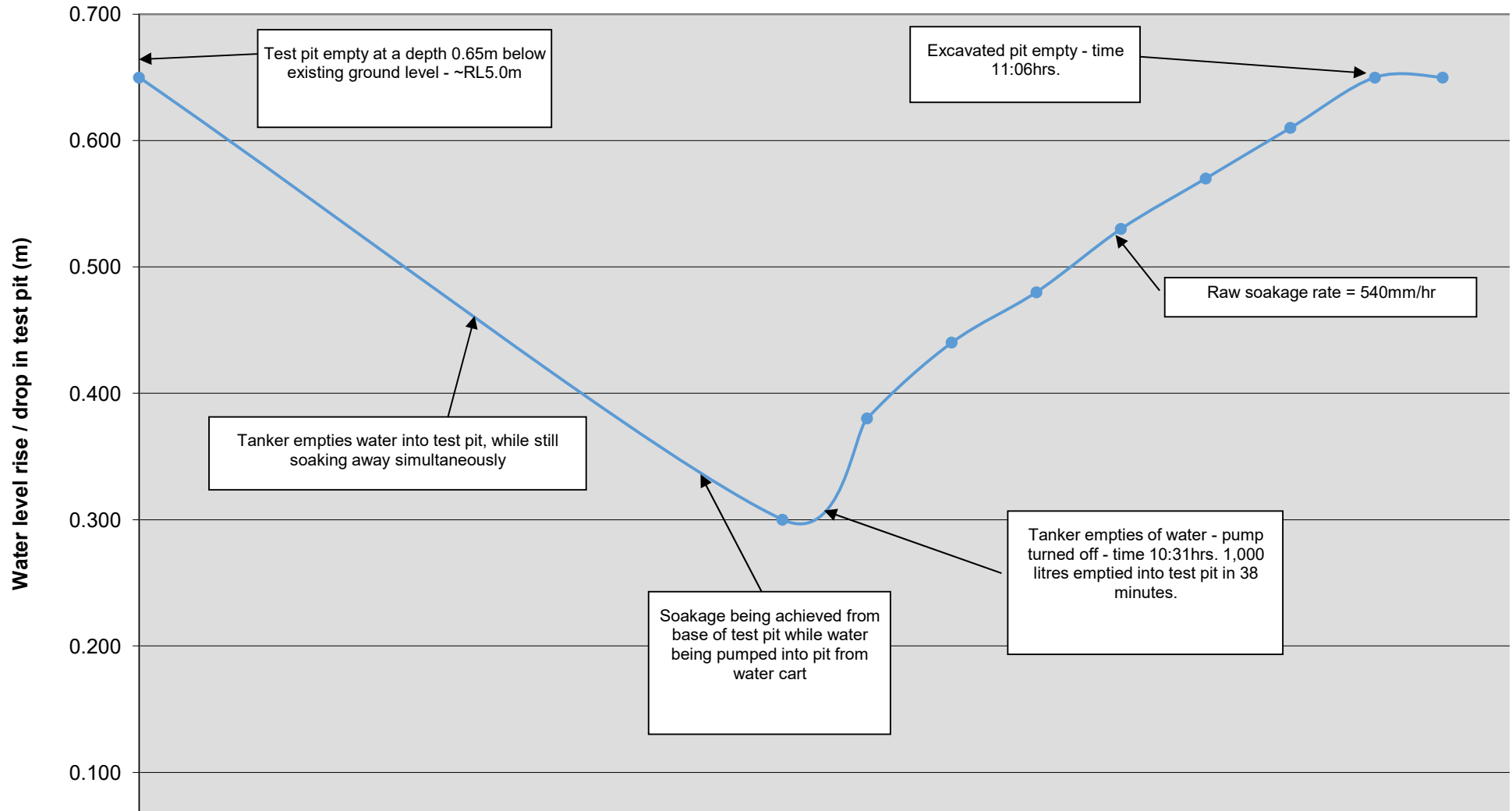
APPENDIX C

Design Drawings

APPENDIX D

Calculations

Plot of water level (m) vs time (hrs) - Location A - 08 July 2024
(~5.0m = existing ground level)



Job number: 23333
Project name: 160 Mazengarb Road, Paraparaumu
Client: Sussex Trust Ltd
Completed by: JTR

HIRDS V4 Intensity-Duration-Frequency Results

Sitename: Custom Location

Coordinate system: WGS84

Longitude: 175.0045

Latitude: -40.8906

DDF Mode Parameters:

Values: c -0.00538 d 0.44931 e -0.00604 f -0.00321 g 0.243096 h -0.00915 i 2.787368
 Example: Duration (ARI (yrs) x y Rainfall Rate (mm/hr)
 24 100 3.178054 4.600149 5.580525

Rainfall depths (mm) :: Historical Data

| ARI | AEP | 10m | 20m | 30m | 1h | 2h | 6h | 12h | 24h | 48h | 72h | 96h | 120h |
|------|-------|------|------|------|------|------|------|------|------|------|------|------|------|
| 1.58 | 0.633 | 7.25 | 9.88 | 11.9 | 16.2 | 22.1 | 35 | 45.5 | 57.4 | 70.1 | 77.2 | 82 | 85.4 |
| 2 | 0.5 | 7.95 | 10.8 | 13 | 17.7 | 24.1 | 38.1 | 49.4 | 62.4 | 76 | 83.7 | 88.8 | 92.4 |
| 5 | 0.2 | 10.4 | 14.1 | 16.8 | 22.9 | 31 | 48.6 | 62.9 | 79 | 95.9 | 105 | 112 | 116 |
| 10 | 0.1 | 12.2 | 16.5 | 19.8 | 26.8 | 36.1 | 56.5 | 72.8 | 91.2 | 110 | 121 | 128 | 133 |
| 20 | 0.05 | 14.2 | 19.1 | 22.8 | 30.8 | 41.5 | 64.5 | 83 | 104 | 125 | 137 | 145 | 150 |
| 30 | 0.033 | 15.4 | 20.7 | 24.6 | 33.3 | 44.7 | 69.4 | 89.1 | 111 | 134 | 147 | 155 | 161 |
| 40 | 0.025 | 16.2 | 21.8 | 26 | 35.1 | 47 | 72.9 | 93.5 | 117 | 140 | 153 | 162 | 168 |
| 50 | 0.02 | 16.9 | 22.7 | 27.1 | 36.5 | 48.9 | 75.7 | 97 | 121 | 145 | 159 | 167 | 174 |
| 60 | 0.017 | 17.5 | 23.5 | 27.9 | 37.6 | 50.4 | 77.9 | 99.8 | 124 | 149 | 163 | 172 | 178 |
| 80 | 0.013 | 18.4 | 24.7 | 29.3 | 39.5 | 52.8 | 81.5 | 104 | 130 | 156 | 170 | 179 | 186 |
| 100 | 0.01 | 19.1 | 25.6 | 30.4 | 40.9 | 54.7 | 84.4 | 108 | 134 | 161 | 175 | 185 | 191 |
| 250 | 0.004 | 22.1 | 29.6 | 35.1 | 47 | 62.6 | 96 | 122 | 151 | 181 | 197 | 207 | 215 |

Rainfall depths (mm) :: RCP6.0 for the period 2081-2100

| ARI | AEP | 10m | 20m | 30m | 1h | 2h | 6h | 12h | 24h | 48h | 72h | 96h | 120h |
|------|-------|-------|-------|-------|-------|-------|--------|--------|--------|--------|--------|--------|--------|
| 1.58 | 0.633 | 8.65 | 11.8 | 14.2 | 19.4 | 26.1 | 40.3 | 51.4 | 64 | 76.7 | 83.7 | 88.3 | 91.7 |
| 2 | 0.5 | 9.53 | 13 | 15.6 | 21.3 | 28.7 | 44.1 | 56.3 | 69.7 | 83.5 | 91.2 | 96.1 | 99.6 |
| 5 | 0.2 | 12.5 | 17 | 20.4 | 27.7 | 37.2 | 57 | 72.3 | 89.1 | 106 | 116 | 122 | 126 |
| 10 | 0.1 | 14.8 | 20.1 | 24 | 32.5 | 43.6 | 66.4 | 84.1 | 103 | 123 | 134 | 141 | 145 |
| 20 | 0.05 | 17.2 | 23.2 | 27.7 | 37.5 | 50.1 | 76.2 | 96.1 | 118 | 140 | 152 | 159 | 165 |
| 30 | 0.033 | 18.7 | 25.2 | 30 | 40.6 | 54.1 | 82.1 | 103 | 126 | 150 | 163 | 171 | 176 |
| 40 | 0.025 | 19.8 | 26.6 | 31.7 | 42.7 | 56.9 | 86.3 | 109 | 133 | 157 | 170 | 179 | 184 |
| 50 | 0.02 | 20.6 | 27.7 | 33 | 44.5 | 59.2 | 89.6 | 113 | 137 | 163 | 176 | 185 | 191 |
| 60 | 0.017 | 21.3 | 28.6 | 34.1 | 45.9 | 61.1 | 92.4 | 116 | 141 | 167 | 181 | 190 | 196 |
| 80 | 0.013 | 22.5 | 30.1 | 35.8 | 48.2 | 64.1 | 96.7 | 121 | 148 | 175 | 189 | 198 | 204 |
| 100 | 0.01 | 23.4 | 31.3 | 37.2 | 50 | 66.4 | 100 | 126 | 153 | 180 | 195 | 204 | 210 |
| 200 | 0.006 | 25.87 | 34.50 | 41.00 | 54.93 | 72.80 | 109.33 | 136.67 | 166.33 | 195.33 | 211.00 | 220.67 | 227.33 |
| 250 | 0.004 | 27.1 | 36.1 | 42.9 | 57.4 | 76 | 114 | 142 | 173 | 203 | 219 | 229 | 236 |

Rainfall depths (mm) :: RCP8.5 for the period 2081-2100

| ARI | AEP | 10m | 20m | 30m | 1h | 2h | 6h | 12h | 24h | 48h | 72h | 96h | 120h |
|------|-------|-------|-------|-------|-------|-------|--------|--------|--------|--------|--------|--------|--------|
| 1.58 | 0.633 | 9.47 | 12.9 | 15.5 | 21.2 | 28.5 | 43.4 | 54.8 | 67.8 | 80.6 | 87.4 | 91.9 | 95.3 |
| 2 | 0.5 | 10.4 | 14.2 | 17.1 | 23.3 | 31.3 | 47.7 | 60.3 | 73.9 | 87.9 | 95.5 | 100 | 104 |
| 5 | 0.2 | 13.8 | 18.7 | 22.4 | 30.5 | 40.8 | 61.8 | 77.8 | 94.9 | 112 | 122 | 128 | 132 |
| 10 | 0.1 | 16.4 | 22.1 | 26.4 | 35.8 | 47.9 | 72.2 | 90.7 | 110 | 130 | 141 | 148 | 153 |
| 20 | 0.05 | 19 | 25.7 | 30.6 | 41.4 | 55.2 | 83 | 104 | 126 | 148 | 160 | 168 | 173 |
| 30 | 0.033 | 20.7 | 27.8 | 33.2 | 44.8 | 59.6 | 89.5 | 112 | 135 | 159 | 172 | 180 | 185 |
| 40 | 0.025 | 21.8 | 29.4 | 35 | 47.2 | 62.7 | 94.2 | 117 | 142 | 167 | 180 | 188 | 194 |
| 50 | 0.02 | 22.8 | 30.6 | 36.5 | 49.2 | 65.3 | 97.7 | 122 | 147 | 173 | 187 | 195 | 200 |
| 60 | 0.017 | 23.6 | 31.6 | 37.7 | 50.7 | 67.3 | 101 | 126 | 151 | 178 | 192 | 200 | 206 |
| 80 | 0.013 | 24.9 | 33.3 | 39.6 | 53.3 | 70.7 | 106 | 131 | 158 | 186 | 200 | 209 | 215 |
| 100 | 0.01 | 25.8 | 34.6 | 41.1 | 55.3 | 73.2 | 109 | 136 | 164 | 192 | 206 | 216 | 221 |
| 200 | 0.006 | 28.53 | 38.13 | 45.30 | 60.77 | 80.27 | 119.67 | 148.00 | 178.00 | 208.00 | 223.33 | 233.33 | 239.00 |
| 250 | 0.004 | 29.9 | 39.9 | 47.4 | 63.5 | 83.8 | 125 | 154 | 185 | 216 | 232 | 242 | 248 |

Historical + 20% climate change

| | 10m | 20m | 30m | 1h | 2h | 6h | 12h | 24h |
|--------|-------|-------|-------|-------|-------|--------|-------|--------|
| 10 yr | 14.64 | 19.8 | 23.76 | 32.16 | 43.32 | 67.8 | 87.36 | 109.44 |
| 100 yr | 22.92 | 30.72 | 36.48 | 49.08 | 65.64 | 101.28 | 129.6 | 160.8 |

Comparison with RCP Pathways

| | 10m | 20m | 30m | 1h | 2h | 6h | 12h | 24h |
|---------------|----------|----------|------|----------|----------|----------|----------|----------|
| RCP6.0 - 10yr | 14.8 | 20.1 | 24 | 32.5 | 43.6 | 66.4 | 84.1 | 103 |
| RCP6.0 100yr | 23.4 | 31.3 | 37.2 | 50 | 66.4 | 100 | 126 | 153 |
| RCP6.0 100yr | 25.86667 | 34.5 | 41 | 54.93333 | 72.8 | 109.3333 | 136.6667 | 166.3333 |
| RCP8.5 10 yr | 16.4 | 22.1 | 26.4 | 35.8 | 47.9 | 72.2 | 90.7 | 110 |
| RCP8.5 100 yr | 25.8 | 34.6 | 41.1 | 55.3 | 73.2 | 109 | 136 | 164 |
| RCP8.5 100 yr | 28.53333 | 38.13333 | 45.3 | 60.76667 | 80.26667 | 119.6667 | 148 | 178 |

Interpolated

Interpolated

| | Pre Development | | | | Post Development | | | | |
|--------------|-----------------|-------------|---------------|--------------|------------------|--------------|-------------|------------|--------------|
| | Total | Impermeable | % impermeable | Curve Number | Total | Unattenuated | Attenuated | % Imp | Curve Number |
| Roof | 375 | Yes | 100% | | 2448 | 0 | 2448 | 100% | |
| Road | 0 | No | 0% | | 1114 | 0 | 1114 | 100% | |
| Carpark | 0 | No | 0% | | 809 | 0 | 809 | 100% | |
| Footpath | 0 | No | 0% | | 311 | 0 | 311 | 100% | |
| Paving | 0 | No | 0% | | 1646 | 0 | 1646 | 100% | |
| Refuse | 0 | No | 0% | | 25 | 0 | 25 | 100% | |
| Turf | 0 | No | 0% | | 511 | 0 | 511 | 100% | |
| Lawn | 6795 | No | 0% | | 306 | 0 | 306 | 0% | |
| Total | 7170 | | 5% | 49 | 7170 | 0 | 7170 | 96% | 49 |

Schedule 4 Appendix B: Curve Number Delineation Tables

(Sourced from USACE, 2000)

Table 2-2a – Runoff curve numbers for urban areas^{4,5} (SCS, 1986)

| Cover Description | Curve numbers for hydrologic soil group | | | |
|--|---|----|----|----|
| | A | B | C | D |
| Fully developed urban areas (vegetation established) | | | | |
| Open space (lawns, parks, golf courses, cemeteries etc) ⁶ | | | | |
| Poor condition (Grass cover <50%) | 68 | 79 | 86 | 89 |
| Fair condition (grass cover 50% to 75%) | 49 | 69 | 79 | 84 |
| Good condition (grass cover >75%) | 39 | 61 | 74 | 80 |
| Impervious areas: | | | | |
| Paved parking lots, roofs, driveways, etc. (excluding right-of-way) | 98 | 98 | 98 | 98 |
| Streets and roads: | | | | |
| Paved curbs and storm sewers (excluding right-of-way) | 98 | 98 | 98 | 98 |
| Paved open ditches (including right-of-way) | 83 | 89 | 92 | 93 |
| Gravel (including right-of-way) | 76 | 85 | 89 | 91 |
| Dirt (including right-of-way) | 72 | 82 | 87 | 89 |
| Western desert urban areas: | | | | |
| Natural desert landscaping (pervious areas only) ⁷ | 63 | 77 | 85 | 88 |
| Artificial desert landscaping (impervious weed barrier, desert shrub with 1- to 2-inch sand or gravel mulch and basin borders) | 96 | 96 | 96 | 96 |
| Urban districts: | | | | |
| Commercial and business | 85 | 89 | 92 | 95 |
| Industrial | 72 | 81 | 88 | 93 |
| Residential districts by average lot size: | | | | |
| 1/8 acre or less (town houses) | 65 | 77 | 85 | 90 |
| 1/4 acre | 38 | 61 | 75 | 83 |
| 1/3 acre | 30 | 57 | 72 | 81 |
| 1/2 acre | 25 | 54 | 70 | 80 |
| 1 acre | 20 | 51 | 68 | 79 |
| 2 acres | 12 | 46 | 65 | 77 |
| Developing urban areas | | | | |

Calculation Sheet

| | | | | | |
|----------------------|---------------------------------|------------------|-----------|------------|-----|
| Project/Job: | 160 Mazengarb Road, Paraparaumu | Date: | 25-Jul-24 | | |
| Subject/Task: | HEC HMS Parameters | Job No: | 23333 | By: | JTR |
| Client/Ref: | Sussex Trust Ltd | Sheet No: | 1 | of | 5 |

| | | | | | | | |
|-----------------|--------|--------|--------|--------|---------|---------|---------|
| Storm Durations | 10 min | 20 min | 30 min | 60 min | 120 min | 360 min | 720 min |
| 10% AEP | 14.8 | 20.1 | 24 | 32.5 | 43.6 | 66.4 | 84.1 |
| 1% AEP | 23.4 | 31.3 | 37.2 | 50 | 66.4 | 100 | 126 |

HIRDS v4 RCP6 to 2100 Driveway/Right of way 1

HEC HMS Parameters

| Catchment | Pre Dev | Post Dev | | |
|-------------------------|-----------|------------|--------------|-----------|
| | Site | Attenuated | Unattenuated | Total |
| Area (km2) | 0.00717 | 0.007170 | 0.000000 | 0.007170 |
| Curve Number (weighted) | 49 | 49 | 49 | 49 |
| Area Imp (km2) | 0.0003750 | 0.0068640 | 0.0000000 | 0.0068640 |
| Area Per (km2) | 0.0067950 | 0.0003060 | 0.0000000 | 0.0003060 |
| % impervious | 5.2 | 95.7 | 0.0 | 95.7 |
| St (mm) | 264.4 | | | |
| 0.1St*Ar | 0.2 | | | |
| Ia Impervious (0*Ai) | 0.0000 | 0.0000 | 0.0000 | |
| Ia Pervious (5 x Ap) | 0.0340 | 0.0015 | 0.0000 | |
| Weighted Ia | 4.7 | 0.2 | 0.0 | |
| ToC (mins) | 60 | 10 | 10 | |
| Lag (mins) | 36 | 6 | 6 | |

$$S_t = \left(\frac{1000}{CN} - 10 \right) 25.4$$

[Equation 3]

Table 2-1 Initial abstraction values

| Landuse | Initial Abstraction |
|---|---------------------|
| Undeveloped (i.e. rural, pasture, forestry) | 0.1 S _t |
| Developed pervious | 5 mm |
| Developed impervious | 0 mm |

$$Weighted I_a = \frac{0.1S_{t_r}A_r + 0_iA_i + 5_pA_p}{Total\ area}$$

[Equation 4]

Where I_a is in mm;

Ar = Area of the rural portion of the catchment
 Ai = Impervois portion of the developed catchment
 Ap = Pervious portion of the developed catchment

Soakage System Parameters

Proposed development Soakage System

| | | | |
|------------------------------|-----------|--------|--------------------------------|
| Product: Cirtex Crate System | | Number | |
| Length (L) = | 25.74 m | 36 | (Cirtex Crate Length = 0.715m) |
| Width (W) = | 18.00 m | 45 | (Cirtex Crate Width = 0.400m) |
| Height (H) = | 0.86 m | 2 | (Cirtex system = Double crate) |
| Internal volume = | 398.46 m3 | | |
| Max total storage = | 378.53 m3 | | (Cirtex void ration = 0.95) |

Storage vs Depth Function

| Depth | Elevation | Cummulative Storage (m3) | Storage (1000m3) |
|--------|-----------|--------------------------|------------------|
| 0.0000 | 3.5000 | 0 | 0.000000 |
| 0.0000 | 3.9300 | 191.258496 | 0.191258 |
| 0.0000 | 4.3600 | 378.53244 | 0.378532 |

Calculation Sheet

| | | | | |
|---------------|---------------------------------|-----------|-----------|---------|
| Project/Job: | 160 Mazengarb Road, Paraparaumu | Date: | 25-Jul-24 | |
| Subject/Task: | HEC HMS Parameters | Job No: | 23333 | By: JTR |
| Client/Ref: | Sussex Trust Ltd | Sheet No: | 2 | of 6 |

10% AEP

Global Summary Results for Run "Run 10% AEP with CC Pre"

Project: Cuttriss Example Simulation Run: Run 10% AEP with CC Pre

Start of Run: 01Jan2000, 00:00 Basin Model: Basin 1 - Pre Devp
 End of Run: 02Jan2000, 00:00 Meteorologic Model: Hydrology 01
 Compute Time: 24Jul2024, 13:16:56 Control Specifications: Control 1

Show Elements: All Elements Volume Units: MM 1000 M3 Sorting: Hydrologic

| Hydrologic Element | Drainage Area (KM2) | Peak Discharge (M3/S) | Time of Peak | Volume (1000 M3) |
|--------------------|---------------------|-----------------------|------------------|------------------|
| Pre Devp Site | 0.0071700 | 0.02279 | 01Jan2000, 14:39 | 0.21468 |
| Outlet | 0.0071700 | 0.02279 | 01Jan2000, 14:39 | 0.21468 |

Global Summary Results for Run "Run 10% AEP with CC Post"

Project: Cuttriss Example Simulation Run: Run 10% AEP with CC Post

Start of Run: 01Jan2000, 00:00 Basin Model: Basin 2 - Post Devp
 End of Run: 02Jan2000, 00:00 Meteorologic Model: Hydrology 01
 Compute Time: 24Jul2024, 13:16:53 Control Specifications: Control 1

Show Elements: All Elements Volume Units: MM 1000 M3 Sorting: Hydrologic

| Hydrologic Element | Drainage Area (KM2) | Peak Discharge (M3/S) | Time of Peak | Volume (1000 M3) |
|--------------------|---------------------|-----------------------|------------------|------------------|
| Post Devp Site | 0.0071700 | 0.15152 | 01Jan2000, 14:07 | 0.71730 |
| Soakage System | 0.0071700 | 0.01500 | 01Jan2000, 13:07 | 0.52386 |

Summary Results for Reservoir "Soakage System"

Project: Cuttriss Example Simulation Run: Run 10% AEP with CC Post
 Reservoir: Soakage System

Start of Run: 01Jan2000, 00:00 Basin Model: Basin 2 - Post Devp
 End of Run: 02Jan2000, 00:00 Meteorologic Model: Hydrology 01
 Compute Time: 24Jul2024, 13:16:53 Control Specifications: Control 1

Volume Units: MM 1000 M3

Computed Results

| | | | |
|-------------------|-------------------|------------------------------|-------------------|
| Peak Inflow: | 0.15152 (M3/S) | Date/Time of Peak Inflow: | 01Jan2000, 14:07 |
| Peak Discharge: | 0.01500 (M3/S) | Date/Time of Peak Discharge: | 01Jan2000, 13:07 |
| Inflow Volume: | 0.71730 (1000 M3) | Peak Storage: | 0.19345 (1000 M3) |
| Discharge Volume: | 0.52386 (1000 M3) | Peak Elevation: | 3.94817 (M) |

Calculation Sheet

| | | | | | |
|---------------|---------------------------------|-----------|-----------|-----|-----|
| Project/Job: | 160 Mazengarb Road, Paraparaumu | Date: | 25-Jul-24 | | |
| Subject/Task: | HEC HMS Parameters | Job No: | 23333 | By: | JTR |
| Client/Ref: | Sussex Trust Ltd | Sheet No: | 3 | of | 6 |

1% AEP

Global Summary Results for Run "Run 1% AEP with CC Pre"

Project: Cuttriss Example Simulation Run: Run 1% AEP with CC Pre

Start of Run: 01Jan2000, 00:00 Basin Model: Basin 1 - Pre Devp
 End of Run: 02Jan2000, 00:00 Meteorologic Model: Hydrology 01
 Compute Time: 24Jul2024, 13:08:17 Control Specifications: Control 1

Show Elements: All Elements Volume Units: MM 1000 M3 Sorting: Hydrologic

| Hydrologic Element | Drainage Area (KM2) | Peak Discharge (M3/S) | Time of Peak | Volume (1000 M3) |
|--------------------|---------------------|-----------------------|------------------|------------------|
| Pre Devp Site | 0.0071700 | 0.04569 | 01Jan2000, 14:39 | 0.41137 |
| Outlet | 0.0071700 | 0.04569 | 01Jan2000, 14:39 | 0.41137 |

Global Summary Results for Run "Run 1% AEP with CC Post"

Project: Cuttriss Example Simulation Run: Run 1% AEP with CC Post

Start of Run: 01Jan2000, 00:00 Basin Model: Basin 2 - Post Devp
 End of Run: 02Jan2000, 00:00 Meteorologic Model: Hydrology 01
 Compute Time: 24Jul2024, 13:08:10 Control Specifications: Control 1

Show Elements: All Elements Volume Units: MM 1000 M3 Sorting: Hydrologic

| Hydrologic Element | Drainage Area (KM2) | Peak Discharge (M3/S) | Time of Peak | Volume (1000 M3) |
|--------------------|---------------------|-----------------------|------------------|------------------|
| Post Devp Site | 0.0071700 | 0.23977 | 01Jan2000, 14:07 | 1.06890 |
| Soakage System | 0.0071700 | 0.01500 | 01Jan2000, 11:12 | 0.69333 |

Summary Results for Reservoir "Soakage System"

Project: Cuttriss Example Simulation Run: Run 1% AEP with CC Post
 Reservoir: Soakage System

Start of Run: 01Jan2000, 00:00 Basin Model: Basin 2 - Post Devp
 End of Run: 02Jan2000, 00:00 Meteorologic Model: Hydrology 01
 Compute Time: 24Jul2024, 13:08:10 Control Specifications: Control 1

Volume Units: MM 1000 M3

Computed Results

| | | | |
|-------------------|-------------------|------------------------------|-------------------|
| Peak Inflow: | 0.23977 (M3/S) | Date/Time of Peak Inflow: | 01Jan2000, 14:07 |
| Peak Discharge: | 0.01500 (M3/S) | Date/Time of Peak Discharge: | 01Jan2000, 11:12 |
| Inflow Volume: | 1.06890 (1000 M3) | Peak Storage: | 0.37557 (1000 M3) |
| Discharge Volume: | 0.69333 (1000 M3) | Peak Elevation: | 4.35997 (M) |

Calculation Sheet

| | | | | | |
|---------------|---------------------------------|-----------|-----------|-----|-----|
| Project/Job: | 160 Mazengarb Road, Paraparaumu | Date: | 25-Jul-24 | | |
| Subject/Task: | HEC HMS Parameters | Job No: | 23333 | By: | JTR |
| Client/Ref: | Sussex Trust Ltd | Sheet No: | 4 | of | 6 |

0.5% AEP

Global Summary Results for Run "Run 0.5% AEP with CC Pre"

Project: Cuttriss Example Simulation Run: Run 0.5% AEP with CC Pre

Start of Run: 01Jan2000, 00:00 Basin Model: Basin 1 - Pre Devp
 End of Run: 02Jan2000, 00:00 Meteorologic Model: Hydrology 01
 Compute Time: 25Jul2024, 10:07:04 Control Specifications: Control 1

Show Elements: Volume Units: MM 1000 M3 Sorting:

| Hydrologic Element | Drainage Area (KM2) | Peak Discharge (M3/S) | Time of Peak | Volume (1000 M3) |
|--------------------|---------------------|-----------------------|------------------|------------------|
| Pre Devp Site | 0.0071700 | 0.05286 | 01Jan2000, 14:39 | 0.47122 |
| Outlet | 0.0071700 | 0.05286 | 01Jan2000, 14:39 | 0.47122 |

Global Summary Results for Run "Run 0.5% AEP with CC Post"

Project: Cuttriss Example Simulation Run: Run 0.5% AEP with CC Post

Start of Run: 01Jan2000, 00:00 Basin Model: Basin 2 - Post Devp
 End of Run: 02Jan2000, 00:00 Meteorologic Model: Hydrology 01
 Compute Time: 25Jul2024, 10:07:00 Control Specifications: Control 1

Show Elements: Volume Units: MM 1000 M3 Sorting:

| Hydrologic Element | Drainage Area (KM2) | Peak Discharge (M3/S) | Time of Peak | Volume (1000 M3) |
|--------------------|---------------------|-----------------------|------------------|------------------|
| Post Devp Site | 0.0071700 | 0.26514 | 01Jan2000, 14:07 | 1.16497 |
| Soakage System | 0.0071700 | 0.01500 | 01Jan2000, 11:10 | 0.72381 |

Summary Results for Reservoir "Soakage System"

Project: Cuttriss Example Simulation Run: Run 0.5% AEP with CC Post
 Reservoir: Soakage System

Start of Run: 01Jan2000, 00:00 Basin Model: Basin 2 - Post Devp
 End of Run: 02Jan2000, 00:00 Meteorologic Model: Hydrology 01
 Compute Time: 25Jul2024, 10:07:00 Control Specifications: Control 1

Volume Units: MM 1000 M3

Computed Results

| | | | |
|-------------------|-------------------|------------------------------|-------------------|
| Peak Inflow: | 0.26514 (M3/S) | Date/Time of Peak Inflow: | 01Jan2000, 14:07 |
| Peak Discharge: | 0.01500 (M3/S) | Date/Time of Peak Discharge: | 01Jan2000, 11:10 |
| Inflow Volume: | 1.16497 (1000 M3) | Peak Storage: | 0.44119 (1000 M3) |
| Discharge Volume: | 0.72381 (1000 M3) | Peak Elevation: | 4.35504 (M) |

Calculation Sheet

| | | | | | |
|---------------|---------------------------------|-----------|-----------|-----|-----|
| Project/Job: | 160 Mazengarb Road, Paraparaumu | Date: | 25-Jul-24 | | |
| Subject/Task: | HEC HMS Parameters | Job No: | 23333 | By: | JTR |
| Client/Ref: | Sussex Trust Ltd | Sheet No: | 5 | of | 6 |

| Storm Durations | 10 min | 20 min | 30 min | 60 min | 120 min | 360 min | 720 min | 1440 min | |
|-----------------|--------|--------|--------|--------|---------|---------|---------|----------|-----------------------|
| 10% AEP | 14.8 | 20.1 | 24 | 32.5 | 43.6 | 66.4 | 84.1 | 103 | HIRDS v4 RCP6 to 2100 |
| 1% AEP | 23.4 | 31.3 | 37.2 | 50 | 66.4 | 100 | 126 | 153 | HIRDS v4 RCP6 to 2101 |

| Catchment Area | m2 | % Impermeable | Effective Area |
|------------------|------|---------------|----------------|
| Pre Development | 7170 | 0.05 | 375 |
| Post Development | 7170 | 0.96 | 6864 |

| Nested Profile | | 10% AEP Rainfall Depth | Pre Dev Volume 10% | Post Dev Volume 10% | 1% AEP Rainfall Depth | Pre Dev Volume 1% | Post Dev Volume 10% |
|----------------|----------|------------------------------|--------------------|---------------------|-----------------------|-------------------|---------------------|
| Start Time | End Time | | | | | | |
| 0:00 | 0:05 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 0:05 | 0:10 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 0:10 | 0:15 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 0:15 | 0:20 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 0:20 | 0:25 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 0:25 | 0:30 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 0:30 | 0:35 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 0:35 | 0:40 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 0:40 | 0:45 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 0:45 | 0:50 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 0:50 | 0:55 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 0:55 | 1:00 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 1:00 | 1:05 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 1:05 | 1:10 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 1:10 | 1:15 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 1:15 | 1:20 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 1:20 | 1:25 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 1:25 | 1:30 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 1:30 | 1:35 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 1:35 | 1:40 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 1:40 | 1:45 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 1:45 | 1:50 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 1:50 | 1:55 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 1:55 | 2:00 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 2:00 | 2:05 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 2:05 | 2:10 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 2:10 | 2:15 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 2:15 | 2:20 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 2:20 | 2:25 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 2:25 | 2:30 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 2:30 | 2:35 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 2:35 | 2:40 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 2:40 | 2:45 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 2:45 | 2:50 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 2:50 | 2:55 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 2:55 | 3:00 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 3:00 | 3:05 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 3:05 | 3:10 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 3:10 | 3:15 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 3:15 | 3:20 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 3:20 | 3:25 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 3:25 | 3:30 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 3:30 | 3:35 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 3:35 | 3:40 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 3:40 | 3:45 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 3:45 | 3:50 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 3:50 | 3:55 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 3:55 | 4:00 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 4:00 | 4:05 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 4:05 | 4:10 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 4:10 | 4:15 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 4:15 | 4:20 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 4:20 | 4:25 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 4:25 | 4:30 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 4:30 | 4:35 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 4:35 | 4:40 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 4:40 | 4:45 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 4:45 | 4:50 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 4:50 | 4:55 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 4:55 | 5:00 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 5:00 | 5:05 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 5:05 | 5:10 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 5:10 | 5:15 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 5:15 | 5:20 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 5:20 | 5:25 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 5:25 | 5:30 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 5:30 | 5:35 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 5:35 | 5:40 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 5:40 | 5:45 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 5:45 | 5:50 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 5:50 | 5:55 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 5:55 | 6:00 | 0.007 of 24 hr minus 12 hour | 0.1323 | 0.05 | 0.91 | 0.19 | 1.30 |
| 6:00 | 6:05 | 0.014 of 12 hr minus 6 hour | 0.2478 | 0.09 | 1.70 | 0.36 | 2.50 |
| 6:05 | 6:10 | 0.014 of 12 hr minus 6 hour | 0.2478 | 0.09 | 1.70 | 0.36 | 2.50 |
| 6:10 | 6:15 | 0.014 of 12 hr minus 6 hour | 0.2478 | 0.09 | 1.70 | 0.36 | 2.50 |
| 6:15 | 6:20 | 0.014 of 12 hr minus 6 hour | 0.2478 | 0.09 | 1.70 | 0.36 | 2.50 |
| 6:20 | 6:25 | 0.014 of 12 hr minus 6 hour | 0.2478 | 0.09 | 1.70 | 0.36 | 2.50 |
| 6:25 | 6:30 | 0.014 of 12 hr minus 6 hour | 0.2478 | 0.09 | 1.70 | 0.36 | 2.50 |
| 6:30 | 6:35 | 0.014 of 12 hr minus 6 hour | 0.2478 | 0.09 | 1.70 | 0.36 | 2.50 |
| 6:35 | 6:40 | 0.014 of 12 hr minus 6 hour | 0.2478 | 0.09 | 1.70 | 0.36 | 2.50 |
| 6:40 | 6:45 | 0.014 of 12 hr minus 6 hour | 0.2478 | 0.09 | 1.70 | 0.36 | 2.50 |
| 6:45 | 6:50 | 0.014 of 12 hr minus 6 hour | 0.2478 | 0.09 | 1.70 | 0.36 | 2.50 |
| 6:50 | 6:55 | 0.014 of 12 hr minus 6 hour | 0.2478 | 0.09 | 1.70 | 0.36 | 2.50 |
| 6:55 | 7:00 | 0.014 of 12 hr minus 6 hour | 0.2478 | 0.09 | 1.70 | 0.36 | 2.50 |

Calculation Sheet

| | | | | | |
|---------------|---------------------------------|-----------|-----------|-----|-----|
| Project/Job: | 160 Mazengarb Road, Paraparaumu | Date: | 25-Jul-24 | | |
| Subject/Task: | HEC HMS Parameters | Job No: | 23333 | By: | JTR |
| Client/Ref: | Sussex Trust Ltd | Sheet No: | 6 | of | 6 |

| Storm Durations | 10 min | 20 min | 30 min | 60 min | 120 min | 360 min | 720 min | 1440 min | |
|-----------------|----------|--------|--------|------------|---------|-------------|-------------|-------------|-----------------------|
| 1% AEP | 23.4 | 31.3 | 37.2 | 50 | 66.4 | 100 | 126 | 153 | HIRDS v4 RCP6 to 2100 |
| 0.5% AEP | 25.86667 | 34.5 | 41 | 54.9333333 | 72.8 | 109.3333333 | 136.6666667 | 166.3333333 | HIRDS v4 RCP6 to 2101 |

| Catchment Area | m2 | % Impermeable | Effective Area |
|------------------|------|---------------|----------------|
| Pre Development | 7170 | 0.05 | 375 |
| Post Development | 7170 | 0.96 | 6864 |

| Nested Profile | | 1% AEP Rainfall | Pre Dev Volume | Post Dev Volume | 0.5% AEP | Pre Dev Volume | Post Dev | |
|----------------|----------|------------------------------|----------------|-----------------|----------------|----------------|-------------|------|
| Start Time | End Time | Depth | 1% | 1% | Rainfall Depth | 0.5% | Volume 0.5% | |
| 0:00 | 0:05 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 0:05 | 0:10 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 0:10 | 0:15 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 0:15 | 0:20 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 0:20 | 0:25 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 0:25 | 0:30 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 0:30 | 0:35 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 0:35 | 0:40 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 0:40 | 0:45 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 0:45 | 0:50 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 0:50 | 0:55 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 0:55 | 1:00 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 1:00 | 1:05 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 1:05 | 1:10 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 1:10 | 1:15 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 1:15 | 1:20 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 1:20 | 1:25 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 1:25 | 1:30 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 1:30 | 1:35 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 1:35 | 1:40 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 1:40 | 1:45 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 1:45 | 1:50 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 1:50 | 1:55 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 1:55 | 2:00 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 2:00 | 2:05 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 2:05 | 2:10 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 2:10 | 2:15 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 2:15 | 2:20 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 2:20 | 2:25 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 2:25 | 2:30 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 2:30 | 2:35 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 2:35 | 2:40 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 2:40 | 2:45 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 2:45 | 2:50 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 2:50 | 2:55 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 2:55 | 3:00 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 3:00 | 3:05 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 3:05 | 3:10 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 3:10 | 3:15 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 3:15 | 3:20 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 3:20 | 3:25 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 3:25 | 3:30 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 3:30 | 3:35 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 3:35 | 3:40 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 3:40 | 3:45 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 3:45 | 3:50 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 3:50 | 3:55 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 3:55 | 4:00 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 4:00 | 4:05 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 4:05 | 4:10 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 4:10 | 4:15 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 4:15 | 4:20 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 4:20 | 4:25 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 4:25 | 4:30 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 4:30 | 4:35 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 4:35 | 4:40 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 4:40 | 4:45 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 4:45 | 4:50 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 4:50 | 4:55 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 4:55 | 5:00 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 5:00 | 5:05 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 5:05 | 5:10 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 5:10 | 5:15 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 5:15 | 5:20 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 5:20 | 5:25 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 5:25 | 5:30 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 5:30 | 5:35 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 5:35 | 5:40 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 5:40 | 5:45 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 5:45 | 5:50 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 5:50 | 5:55 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 5:55 | 6:00 | 0.007 of 24 hr minus 12 hour | 0.189 | 0.07 | 1.30 | 0.21 | 0.08 | 1.43 |
| 6:00 | 6:05 | 0.014 of 12 hr minus 6 hour | 0.364 | 0.14 | 2.50 | 0.38 | 0.14 | 2.63 |
| 6:05 | 6:10 | 0.014 of 12 hr minus 6 hour | 0.364 | 0.14 | 2.50 | 0.38 | 0.14 | 2.63 |
| 6:10 | 6:15 | 0.014 of 12 hr minus 6 hour | 0.364 | 0.14 | 2.50 | 0.38 | 0.14 | 2.63 |
| 6:15 | 6:20 | 0.014 of 12 hr minus 6 hour | 0.364 | 0.14 | 2.50 | 0.38 | 0.14 | 2.63 |
| 6:20 | 6:25 | 0.014 of 12 hr minus 6 hour | 0.364 | 0.14 | 2.50 | 0.38 | 0.14 | 2.63 |
| 6:25 | 6:30 | 0.014 of 12 hr minus 6 hour | 0.364 | 0.14 | 2.50 | 0.38 | 0.14 | 2.63 |
| 6:30 | 6:35 | 0.014 of 12 hr minus 6 hour | 0.364 | 0.14 | 2.50 | 0.38 | 0.14 | 2.63 |
| 6:35 | 6:40 | 0.014 of 12 hr minus 6 hour | 0.364 | 0.14 | 2.50 | 0.38 | 0.14 | 2.63 |
| 6:40 | 6:45 | 0.014 of 12 hr minus 6 hour | 0.364 | 0.14 | 2.50 | 0.38 | 0.14 | 2.63 |
| 6:45 | 6:50 | 0.014 of 12 hr minus 6 hour | 0.364 | 0.14 | 2.50 | 0.38 | 0.14 | 2.63 |
| 6:50 | 6:55 | 0.014 of 12 hr minus 6 hour | 0.364 | 0.14 | 2.50 | 0.38 | 0.14 | 2.63 |
| 6:55 | 7:00 | 0.014 of 12 hr minus 6 hour | 0.364 | 0.14 | 2.50 | 0.38 | 0.14 | 2.63 |

APPENDIX E

Designgroup Stapleton Elliot

DESIGNGROUP
STAPLETON ELLIOTT

designgroupstapletonelliott.co.nz

THAMES PACIFIC

160 MAZENGARB ROAD, PARAPARAUMU
RESOURCE CONSENT

ARCHITECTURAL DRAWINGS

24 JULY 2024



designgroup
stapleton elliott

SITE INFORMATION

Address: 160 Mazengarb Road, Paraparaumu
Legal Description: LOT 12 DP 90944
District Plan Zone: General Residential Zone
Site Area: 7168.6m²



RC02 REV.B

CONTEXT PLAN

THAMES PACIFIC

160 Mazengarb Road, Paraparaumu,
WELLINGTON, 5032

RESOURCE CONSENT

Contractors shall verify all dimensions on site before commencing work. Do not scale from the drawings. If in doubt ask. Copyright of this drawing is vested in Designgroup Stapleton Elliott.

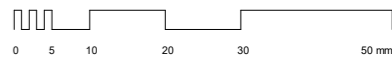
PROJECT No. **PROJECT NUMBER**
PLOT DATE: **24/07/2024 10:49:50 AM**

| NO. | DESCRIPTION | DATE |
|-----|----------------------|------------|
| B | RC Drawings 02 | 2024/07/24 |
| A | RC Drawings | 2024/07/22 |
| 8 | Draft RC drawings 02 | 2024/07/18 |
| 4 | Draft RC drawings | 2024/07/04 |

Site Information

Rainfall Intensity: 60 mm/h
Climate Zone: 3
Corrosion Zone: Zone C
Legal Description: Lot 12 DP 90944
Wind Zone: High

NZBC E2 Compliance: Compliance with NZBC E2 is by means of NZBC E2 AS1. Refer Risk Matrix provided.



A3 Print Scale 1 : 1
A1 Print Scale 1 : 0.5

| | | |
|------------------|----------------|---------------|
| Wellington | +64 4 920 0032 | wn@dgse.co.nz |
| Palmerston North | +64 6 357 4534 | pn@dgse.co.nz |
| Tauranga | +64 7 925 6238 | tr@dgse.co.nz |
| Napier | +64 6 835 6173 | np@dgse.co.nz |
| Auckland | +64 9 976 8288 | ak@dgse.co.nz |



designgroup
stapleton elliott

MASTERPLAN

SITE INFORMATION

Address: 160 Mazengarb Road, Paraparaumu
 Legal Description: LOT 12 DP 90944
 District Plan Zone: General Residential Zone
 Site Area: 7168.6m²

| UNITS | |
|---|-----------|
| Unit A (1Bed) or Unit B1 (2Bed) or Unit D (1Bed Acc) or Unit F (1Bed) | 17 |
| Unit C (3Bed) | 5 |
| Unit E1 (1Bed) or E2 (2Bed) | 13 |
| Unit B2 (2Bed) | 6 |
| TOTAL | 41 |

| CARPARKS | |
|--------------------------|-----------|
| UNIT ADJACENT CARPARK | 29 |
| SITE CARPARK (ALLOCATED) | 12 |
| ACCESSIBLE / GUEST | 2 |
| TOTAL | 43 |



RC04 REV.B

MASTERPLAN

THAMES PACIFIC

160 Mazengarb Road, Paraparaumu,
 WELLINGTON, 5032

RESOURCE CONSENT

Contractors shall verify all dimensions on site before commencing work. Do not scale from the drawings. If in doubt ask. Copyright of this drawing is vested in Designgroup Stapleton Elliott.

PROJECT No. PROJECT NUMBER
 PLOT DATE: 24/07/2024 10:49:59 AM

| NO. | DESCRIPTION | DATE |
|-----|----------------------|------------|
| B | RC Drawings 02 | 2024/07/24 |
| A | RC Drawings | 2024/07/22 |
| 8 | Draft RC drawings 02 | 2024/07/18 |
| 6 | Unit C Placement | 2024/07/17 |
| 5 | HIRTB Sections | 2024/07/15 |
| 4 | Draft RC drawings | 2024/07/04 |
| 2 | Traffic Review | 2024/06/25 |
| 1 | HIRTB Sections | 2024/06/24 |

Site Information

Rainfall Intensity: 60 mm/h
 Climate Zone: 3
 Corrosion Zone: Zone C
 Legal Description: Lot 12 DP 90944
 Wind Zone: High
 NZBC E2 Compliance: Compliance with NZBC E2 is by means of NZBC E2 AS1. Refer Risk Matrix provided.



| | | |
|------------------|----------------|---------------|
| Wellington | +64 4 920 0032 | wn@dgse.co.nz |
| Palmerston North | +64 6 357 4534 | pn@dgse.co.nz |
| Tauranga | +64 7 925 6238 | tr@dgse.co.nz |
| Napier | +64 6 835 6173 | np@dgse.co.nz |
| Auckland | +64 9 976 8288 | ak@dgse.co.nz |

APPENDIX F

Torlesse Ltd



Legend

- ▲ CPT
- Test Pit
- Property Boundary

Client:
Sussex Trust

Project Name:
Geotechnical Assessment Report

Site Location:
160 Mazengarb Road, Paraparaumu

Sketch Title:
Test Location Plan

Project/Report No:
T0399/02

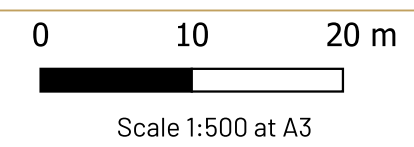
Sketch ID:
1 of 2

| | |
|---------------------|-----------------|
| Author: L Heaton | Initials: LH |
|---------------------|-----------------|

| | |
|--------------------------|-----------------|
| Checked By: N Clendon | Initials: NC |
|--------------------------|-----------------|

| Rev. | Date | Description | Initials |
|------|----------|-------------|----------|
| A | 17/07/24 | Final | LH |
| | | | |
| | | | |
| | | | |

Notes:



Data Courtesy:
LINZ, OPENMAPS



Torlesse™





- Legend**
- ▲ CPT
 - Test Pit
 - Property Boundary

Client:
Sussex Trust

Project Name:
Geotechnical Assessment Report

Site Location:
160 Mazengarb Road, Paraparaumu

Sketch Title:
Proposed Development Plan

Project/Report No:
T0399/02

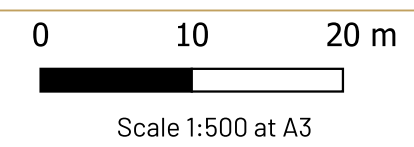
Sketch ID:
2 of 2

| | | | |
|---------|----------|-----------|----|
| Author: | L Heaton | Initials: | LH |
|---------|----------|-----------|----|

| | | | |
|-------------|-----------|-----------|----|
| Checked By: | N Clendon | Initials: | NC |
|-------------|-----------|-----------|----|

| Rev. | Date | Description | Initials |
|------|----------|-------------|----------|
| A | 17/07/24 | Final | LH |
| | | | |
| | | | |
| | | | |
| | | | |

Notes:



Data Courtesy:
LINZ, OPENMAPS



Torlesse™



APPENDIX C: INVESTIGATION RECORDS

Test Pit & DCP Log No: TP01

Client: **Sussex Trust**
 Job No.: **T0399**
 Job Name: **160 Mazengarb Road, Paraparaumu**

Logged by: **LH** Co-ordinates **WGS 84** Contractor: **CPT Elite** Elevation: **6.5 m** Page No:
 Entered by: **LH** Plunge (Degrees): **90** Start Date: **2024-07-08** Northing: **-40.89057**
 Reviewed by: **NC** Trend (Degrees): **0** End Date: **2024-07-08** Easting: **175.00469** **1 of 1**

| Depth Scale | Lithologic Description | Symbol | Samples | Vane Shear Test (Su) | Pocket Penetrometer | Manual DCP | Comments / Additional Notes |
|-------------|---|--------|---------|----------------------|---------------------|------------|-----------------------------|
| 0 | Ground Surface | | | | | | |
| 0 | TOPSOIL | | | | | | |
| | Fine to coarse SAND Light brown, loose, moist. | | | | | | |
| | Fine to medium SAND With trace silt. Dark brown, loose to medium dense, moist. Silt is non-plastic. | | | | | | |
| | Fine SAND Light brown, medium dense, moist. | | | | | | |
| | Fine to medium SAND Grey, medium dense, moist. | | | | | | |
| 2 | From 2.2m bgl - Medium dense to dense. | | | | | | |
| | From 2.5m bgl - Dense. | | | | | | |
| 3 | Test Hole Terminated at 3 m - Target depth. | | | | | | |
| 4 | | | | | | | |
| 5 | | | | | | | |

Test Location Information:

Equipment: **Excavator**
 Size (m): **0.8 x 2.7m**
 Water Level (m):
 Water Level (Elv):

Test Location Notes:

Coordinates obtained from mobile phone GPS.
Elevation estimated from Cuttriss Consultants Topographic Survey.
Groundwater was not encountered.



TP01 Pit



TP01 Stockpile

Test Pit & DCP Log No: TP02

Client: **Sussex Trust**
 Job No.: **T0399**
 Job Name: **160 Mazengarb Road, Paraparaumu**

| | | | | |
|------------------------|-----------------------------|-------------------------------|----------------------------|---------------|
| Logged by: LH | Co-ordinates WGS 84 | Contractor: CPT Elite | Elevation: 6 m | Page No: |
| Entered by: LH | Plunge (Degrees): 90 | Start Date: 2024-07-08 | Northing: -40.89052 | 1 of 1 |
| Reviewed by: NC | Trend (Degrees): 0 | End Date: 2024-07-08 | Easting: 175.00427 | |

| Depth Scale | Lithologic Description | Symbol | Samples | Vane Shear Test (Su) | Pocket Penetrometer | Manual DCP | | | | | Comments / Additional Notes | |
|-------------|---|--------|---------|----------------------|---------------------|------------|---|----|----|----|-----------------------------|--|
| | | | | | | 0 | 5 | 10 | 15 | 20 | | |
| 0 | Ground Surface | | | | | | | | | | | |
| 0 | TOPSOIL | | | | | | | | | | | |
| 0 | Fine to medium SAND With trace gravel. Dark brown, loose, dry to moist. Gravels are fine to medium, sub-round. (FILL?). | | | | | | | | | | | |
| 1 | Fine to medium SAND With trace rootlets. Brown, loose, dry to moist. | | | | | | | | | | | |
| 1 | Fine SAND Light brown, medium dense, dry to moist. | | | | | | | | | | | |
| 2 | Fine to coarse SAND Grey brown, medium dense to dense, moist. From 2.1m bgl - Dense. | | | | | | | | | | | |
| 3 | Test Hole Terminated at 3.1 m - Target depth. | | | | | | | | | | | |
| 4 | | | | | | | | | | | | |
| 5 | | | | | | | | | | | | |

Test Location Information:

Equipment: **Excavator**
 Size (m): **0.8 x 2.7m**
 Water Level (m):
 Water Level (Elv):

Test Location Notes:

Coordinates obtained from mobile phone GPS.
 Elevation estimated from Cuttriss Consultants Topographic Survey.
 Groundwater was not encountered.



TP02 Pit



TP02 Stockpile

Test Pit & DCP Log No: TP03

Client: **Sussex Trust**
 Job No.: **T0399**
 Job Name: **160 Mazengarb Road, Paraparaumu**

Logged by: **LH** Co-ordinates **WGS 84** Contractor: **CPT Elite** Elevation: **5 m** Page No:
 Entered by: **LH** Plunge (Degrees): **90** Start Date: **2024-07-08** Northing: **-40.89075** **1 of 1**
 Reviewed by: **NC** Trend (Degrees): **0** End Date: **2024-07-08** Easting: **175.00415**

| Depth Scale | Lithologic Description | Symbol | Samples | Vane Shear Test (Su) | Pocket Penetrometer | Manual DCP | Comments / Additional Notes |
|-------------|---|--------|---------|----------------------|---------------------|------------|-----------------------------|
| 0 | Ground Surface | | | | | | |
| 0 | TOPSOIL | | | | | | |
| 0 | Fine SAND With minor tree roots. Brown, loose to medium dense, dry. | | | | | | |
| 0 | Fine to coarse SAND With trace rootlets. Light brown, medium dense, dry to moist. | | | | | | |
| 1 | From 1.2m bgl - Medium dense to dense. | | | | | | |
| 2 | From 1.8m bgl - Dense. | | | | | | |
| 2 | From 2.0m bgl - Moist. | | | | | | |
| 3 | Fine to coarse SAND Grey, dense, moist to wet. | | | | | | |
| 3.2 | Test Hole Terminated at 3.2 m - Target depth and pit wall collapse. | | | | | | |

Test Location Information:

Equipment: **Excavator**
 Size (m): **0.8 x 2.7m**
 Water Level (m):
 Water Level (Elv):

Test Location Notes:

Coordinates obtained from mobile phone GPS.
Elevation estimated from Cuttriss Consultants Topographic Survey.
Groundwater was not encountered.



TP03 Pit



TP03 Stockpile

Test Pit & DCP Log No: TP04

Client: **Sussex Trust**
 Job No.: **T0399**
 Job Name: **160 Mazengarb Road, Paraparaumu**

Logged by: **LH** Co-ordinates **WGS 84** Contractor: **CPT Elite** Elevation: **6.5 m** Page No:
 Entered by: **LH** Plunge (Degrees): **90** Start Date: **2024-07-08** Northing: **-40.89083** **1 of 1**
 Reviewed by: **NC** Trend (Degrees): **0** End Date: **2024-07-08** Easting: **175.00359**

| Depth Scale | Lithologic Description | Symbol | Samples | Vane Shear Test (Su) | Pocket Penetrometer | Manual DCP | Comments / Additional Notes |
|-------------|--|--------|---------|----------------------|---------------------|------------|-----------------------------|
| 0 | Ground Surface | | | | | | |
| 0 | TOPSOIL Fine to medium SAND Light brown, loose to medium dense, moist. | | | | | | |
| 0.5 | Fine to coarse SAND Light brown, loose, moist. | | | | | | |
| 1 | From 1.1m bgl - Medium dense. | | | | | | |
| 2 | From 2.0m bgl - Dense. | | | | | | |
| 3 | Test Hole Terminated at 3 m - Target depth. | | | | | | |
| 4 | | | | | | | |
| 5 | | | | | | | |

Test Location Information:

Equipment: **Excavator**
 Size (m): **0.8 x 2.7m**
 Water Level (m):
 Water Level (Elv):

Test Location Notes:

Coordinates obtained from mobile phone GPS.
Elevation estimated from Cuttriss Consultants Topographic Survey.
Groundwater was not encountered.



TP04 Pit



TP04 Stockpile

Test Pit & DCP Log No: TP05

Client: **Sussex Trust**
 Job No.: **T0399**
 Job Name: **160 Mazengarb Road, Paraparaumu**

Logged by: **LH** Co-ordinates **WGS 84** Contractor: **CPT Elite** Elevation: **4.5 m** Page No:
 Entered by: **LH** Plunge (Degrees): **90** Start Date: **2024-07-08** Northing: **-40.89123** **1 of 1**
 Reviewed by: **NC** Trend (Degrees): **0** End Date: **2024-07-08** Easting: **175.00398**

| Depth Scale | Lithologic Description | Symbol | Samples | Vane Shear Test (Su) | Pocket Penetrometer | Manual DCP | | | | | Comments / Additional Notes | |
|-------------|---|--------|---------|----------------------|---------------------|------------|---|----|----|----|-----------------------------|--|
| | | | | | | 0 | 5 | 10 | 15 | 20 | | |
| 0 | Ground Surface | | | | | | | | | | | |
| 0 | TOPSOIL | | | | | | | | | | | |
| | Fine to coarse SAND Light brown, loose, moist. | | | | | | | | | | | |
| | Sandy SILT With trace gravel and rootlets. Dark brown, stiff, moist. Non-plastic. Sand is fine to medium. Gravels are fine, sub-angular. | | | | | | | | | | | |
| | Fine to medium SAND Dark brown, medium dense, moist. | | | | | | | | | | | |
| 1 | Silty fine SAND Grey, moist. Silt has low plasticity. | | | | | | | | | | | |
| | Fine to coarse SAND Grey, medium dense, moist. | | | | | | | | | | | |
| | From 1.8m bgl - Strong organic smell. | | | | | | | | | | | |
| 2 | From 2.5m bgl - Saturated. Organics in base of pit (bark). | | | | | | | | | | | |
| 3 | Test Hole Terminated at 2.6 m - Refusal on organic material (possible log). | | | | | | | | | | | |
| 08 Jul 2024 | | | | | | | | | | | | |
| 3 | | | | | | | | | | | | |
| 4 | | | | | | | | | | | | |
| 5 | | | | | | | | | | | | |

Test Location Information:

Equipment: **Excavator**
 Size (m): **0.8 x 2.7m**
 Water Level (m): **2.6 m**
 Water Level (Elv): **1.9 m**

Test Location Notes:

Coordinates obtained from mobile phone GPS.
 Elevation estimated from Cuttriss Consultants Topographic Survey.



TP05 Pit



TP05 Stockpile

Test Pit & DCP Log No: TP06

Client: **Sussex Trust**
 Job No.: **T0399**
 Job Name: **160 Mazengarb Road, Paraparaumu**

Logged by: **LH** Co-ordinates **WGS 84** Contractor: **CPT Elite** Elevation: **5 m** Page No:
 Entered by: **LH** Plunge (Degrees): **90** Start Date: **2024-07-08** Northing: **-40.89094** **1 of 1**
 Reviewed by: **NC** Trend (Degrees): **0** End Date: **2024-07-08** Easting: **175.0044**

| Depth Scale | Lithologic Description | Symbol | Samples | Vane Shear Test (S _u) | Pocket Penetrometer | Manual DCP | Comments / Additional Notes |
|-------------|--|----------|---------|-----------------------------------|---------------------|------------|-----------------------------|
| 0 | Ground Surface | | | | | | |
| 0 | Fine SAND With trace tree roots. Grey brown, loose to medium dense, dry. | [Symbol] | | | | | |
| | Fine SAND Light brown, medium dense to dense, dry. | [Symbol] | | | | | |
| 1 | From 1.1m bgl - Dense. | [Symbol] | | | | | |
| 2 | Fine to coarse SAND Grey, dense, moist. | [Symbol] | | | | | |
| 3 | From 3.0m bgl - Wet. | [Symbol] | | | | | |
| | Test Hole Terminated at 3.1 m - Target depth and pit wall collapse. | | | | | | |
| 4 | | | | | | | |
| 5 | | | | | | | |

Test Location Information:

Equipment: **Excavator**
 Size (m): **0.8 x 2.7m**
 Water Level (m):
 Water Level (Elv):

Test Location Notes:

Coordinates obtained from mobile phone GPS.
 Elevation estimated from Cuttriss Consultants Topographic Survey.
 Groundwater was not encountered, however some seepage was observed in the base of the pit.



TP06 Pit



TP06 Stockpile